

DIRECTORIO DE PROFESORES DEL CURSO DE OBRAS  
MARITIMAS.- "HIDRAULICA MARITIMA Y DE ESTUA  
RIOS".- DEL 11 DE MARZO AL 3 DE JUNIO. 1985.

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FACULTAD DE INGENIERIA.  
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# EVALUACION DEL PERSONAL DOCENTE

CURSO:

FECHA:

		DOMINIO DEL TEMA	EFICIENCIA EN EL USO DE AYUDAS AUDIO VISUALES	MANTENIMIENTO DEL INTERES. (COMUNICACION CON LOS ASISTENTES, AMENIDAD, FACILIDAD DE EXPRESION).	PUNTUALIDAD
	<b>CONFERENCISTA</b>				
1.	ING. OSCAR FUENTES MARILES				
2.					
3.					
4.					
5.					
6.					
7.					
8.					
9.					

# EVALUACION DE LA ENSEÑANZA

SU EVALUACION SINCERA NOS AYUDARA A MEJORAR LOS PROGRAMAS POSTERIORES QUE DISEÑAREMOS PARA USTED.

TEMA	ORGANIZACION Y DESARROLLO DEL TEMA	GRADO DE PROFUNDIDAD LOGRADO EN EL TEMA	GRADO DE ACTUALIZACION LOGRADO EN EL TEMA	UTILIDAD PRACTICA DEL TEMA	
ONDAS					
TRANSFORMACIONES DE LAS ONDAS					
ONDAS OCEANICAS					
PREDICCIÓN DEL OLEAJE					
EFECTO DEL OLEAJE SOBRE ESTRUCTURAS MARITIMAS.					
PROCESOS COSTEROS					
ESTUARIOS					
ONDAS DE LARGO PERIODO					

## EVALUACION DEL CURSO

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	CONCEPTO	EVALUACION
1.	APLICACION INMEDIATA DE LOS CONCEPTOS EXPUESTOS	
2.	CLARIDAD CON QUE SE EXPUSIERON LOS TEMAS	
3.	GRADO DE ACTUALIZACION LOGRADO CON EL CURSO	
4.	CUMPLIMIENTO DE LOS OBJETIVOS DEL CURSO	
5.	CONTINUIDAD EN LOS TEMAS DEL CURSO	
6.	CALIDAD DE LAS NOTAS DEL CURSO	
7.	GRADO DE MOTIVACION LOGRADO CON EL CURSO	

ESCALA DE EVALUACION DE 1 A 10

1. ¿Qué le pareció el ambiente en la División de Educación Continua?

MUY AGRADABLE	AGRADABLE	DESAGRADABLE

2. Medio de comunicación por el que se enteró del curso:

PERIODICO EXCELSIOR ANUNCIO TITULADO DI VISION DE EDUCACION CONTINUA	PERIODICO NOVEDADES ANUNCIO TITULADO DI VISION DE EDUCACION CONTINUA	FOLLETO DEL CURSO

CARTEL MENSUAL	RADIO UNIVERSIDAD	COMUNICACION CARTA, TELEFONO, VERBAL, ETC.

REVISTAS TECNICAS	FOLLETO ANUAL	CARTELETA UNAM "LOS UNIVERSITARIOS HOY"	GACETA UNAM

3. Medio de transporte utilizado para venir al Palacio de Minería:

AUTOMOVIL PARTICULAR	METRO	OTRO MEDIO

4. ¿Qué cambios haría usted en el programa para tratar de perfeccionar el curso?

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5. ¿Recomendaría el curso a otras personas?

SI	NO

6. ¿Qué cursos le gustaría que ofreciera la División de Educación Continua?

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7. La coordinación académica fue:

EXCELENTE	BUENA	REGULAR	MALA

8. Si está interesado en tomar algún curso intensivo ¿Cuál es el horario más conveniente para usted?

LUNES A VIERNES DE 9 A 13 H. Y DE 14 A 18 H. (CON COMIDAS)	LUNES A VIERNES DE 17 A 21 H.	LUNES, MIERCOLES Y VIERNES DE 18 A 21 H.	MARTES Y JUEVES DE 18 A 21 H.

VIERNES DE 17 A 21 H. SABADOS DE 9 A 14 H.	VIERNES DE 17 A 21 H. SABADOS DE 9 A 13 Y DE 14 A 18 H.	OTRO

9. ¿Qué servicios adicionales desearía que tuviese la División de Educación Continua, para los asistentes?

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10. Otras sugerencias:

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**DIVISION DE EDUCACION CONTINUA  
FACULTAD DE INGENIERIA U.N.A.M.**

CURSO: OBRAS MARITIMAS  
"HIDRAULICA MARITIMA Y DE ESTUARIOS"  
DEL 11 DE MARZO AL 3 DE JUNIO  
MEXICO, D.F.

TRANSFORMACIONES DE LAS ONDAS

ING. OSCAR FUENTES MARILES  
JUNIO 1985

2. TRANSFORMACIONES DE LAS ONDAS

Modificaciones de las ondas progresivas.

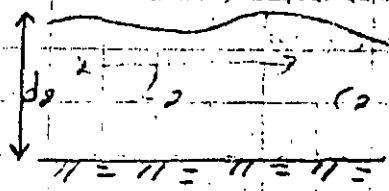
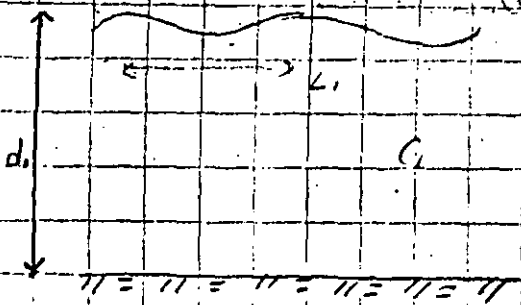
a) Refracción

b) Difracción

c) Reflexión

d) Rompiente

e) Resistencia del fondo



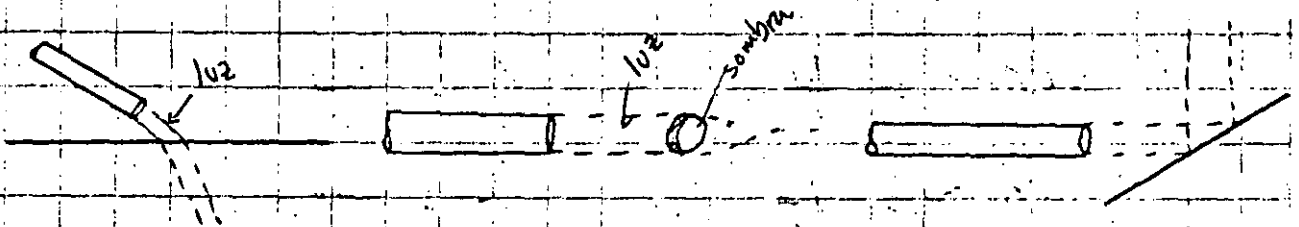
$d_1 > d_2$

$L_1 > L_2$

$c_1 > c_2$

$T_1 = T_2 = T$

\* Analogías con fenómenos de Luz



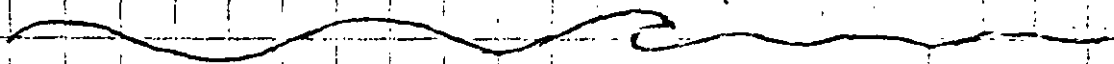
refracción

difracción

reflexión

rompiente

$v > c$





## 2.1 AMORTIGUAMIENTO DEL OLAJE POR LA RESISTENCIA DEL FONDO

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La energía del oleaje en aguas poco profundas disminuye debido a la fricción del fondo, ya que parte de ella se transforma en calor; esto tiene como consecuencia que se amortigue el oleaje.

Sea  $\tau_b$  el esfuerzo cortante en el fondo,  $u_b$  la componente horizontal de la velocidad fuera de una delgada capa adyacente al fondo, entonces la potencia promedio disipada por unidad de área es

$$D = \overline{\tau_b u_b} \quad 1$$

Suponiendo una capa límite turbulenta

$$\tau_b = C_r \rho u_b |u_b| \quad 2$$

en la cual,  $C_r$  es el coeficiente de resistencia (adimensional) que es función de la razón vertical de desplazamiento de las partículas, de la rugosidad de fondo y del número de Reynolds de la capa límite, pero para fines prácticos puede considerarse

$$C_r = 0.01 \quad 3$$

Si se sustituye la ec. A.3.26 y 2 en la 1 se obtiene

$$D = \frac{4}{3\pi} C_r \rho \left( \frac{\sigma a}{\sinh kd} \right)^3 \quad 4$$

Por otra parte, la razón de la energía transferida desde  $x_1$  a  $x_2 = x_1 + \Delta x$  es representada por  $P_1$  y  $P_2$ , donde  $P_2 = P_1 + \frac{dP}{dx} \Delta x$ . La diferencia  $P_1 - P_2$  es igual a la potencia disipada sobre la longitud  $\Delta x$ , la cual es igual a  $D \Delta x$  (por unidad de ancho)

así que  $\frac{dP}{dx} + D = 0 \quad 5$

sustituyendo 4, A.3.52 y A.3.49 se encuentra

$$\rho g n c a \frac{da}{dx} + \frac{4}{3\pi} c_r \rho \left( \frac{\sigma a}{\sinh kd} \right)^3 = 0$$

la cual puede ser escrita como

$$\frac{da}{a^2} + \beta dx = 0$$

siendo

$$\beta = \frac{4}{3\pi} \frac{c_r}{g n c} \left( \frac{\sigma}{\sinh kd} \right)^3$$

o bien usando A.3.23

$$\beta = \frac{4}{3\pi} c_r \frac{k^2}{m \sinh^2 kd \cosh kd}$$

Finalmente la integración de 7 da

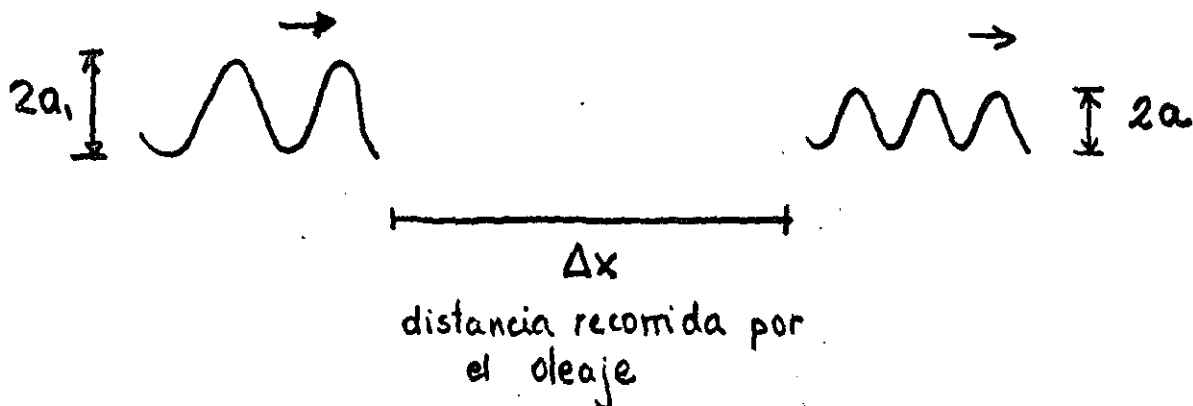
$$\frac{1}{a(x)} - \frac{1}{a(x_1)} = \beta (x - x_1)$$

que también es

$$a = \frac{a_1}{1 + \beta a_1 \Delta x}$$

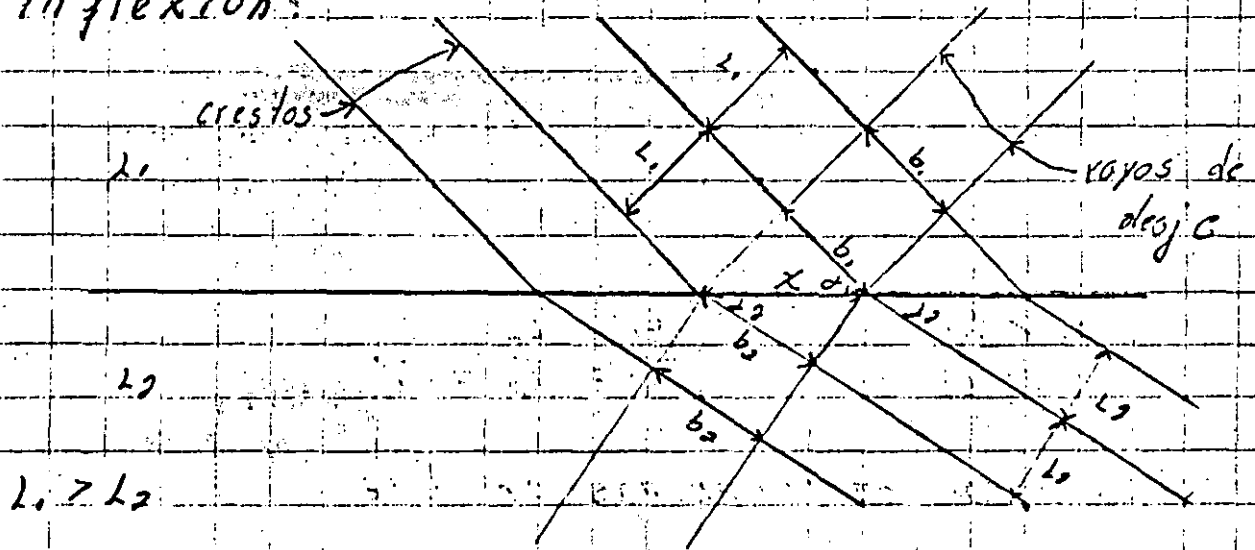
en la cual  $a = a(x)$ ,  $a_1 = a(x_1)$  y  $\Delta x = x - x_1$ .

Por inspección de la ec. 9 se tiene que al medida que disminuye la profundidad, se aproxima a  $\frac{4}{3\pi} \frac{c_r}{d^2}$



## 2.2 Refracción:

Cuando la luz penetra en una sustancia con un ángulo diferente a la normal de su superficie, la luz cambia de dirección produciendo lo que se llama refracción o inflexión.



$$\text{sen } \alpha_1 = \frac{L_1}{x}$$

$$x = \frac{L_1}{\text{sen } \alpha_1}$$

$$\text{sen } \alpha_2 = \frac{L_2}{x}$$

$$x = \frac{L_2}{\text{sen } \alpha_2}$$

Como  $x = x$

$$\frac{L_1}{\text{sen } \alpha_1} = \frac{L_2}{\text{sen } \alpha_2}$$

La expresión anterior se conoce con el nombre de la ley de SNEEL.

$$\text{con } c = \frac{1}{T}$$

$$\frac{c_1}{\text{sen } \alpha_1} = \frac{c_2}{\text{sen } \alpha_2}$$

Potencia promedio en un periodo (por long.) unitario de cresta

$$\bar{P} = \frac{\eta E}{T} = \frac{\eta \gamma H^3 L}{T \cdot 8} = \frac{\eta \gamma H^3 C}{8}$$

La "potencia promedio en un periodo" en bre ortogonales es constante

$$\bar{P}_1 = \bar{P}_2$$

$$\frac{\gamma N_1 H_1^3 C_1}{8} = \frac{\gamma N_2 H_2^3 C_2}{8}$$

En la práctica se parte de la condición de aguas profundas

$$\bar{P}_0 B_0 = \bar{P}_1 B_1 \quad \frac{\gamma N_0 H_0^3 C_0}{8} = \frac{\gamma N_1 H_1^3 C_1}{8}$$

como  $\gamma = \gamma$  y  $N_0 = 0.5$

$$H_1^3 = \frac{0.5 C_0}{N_1 C_1} \frac{B_0}{B_1} H_0^3$$

$$H_1 = \sqrt{\frac{0.5 C_0}{N_1 C_1}} \sqrt{\frac{B_0}{B_1}} H_0$$

$\sqrt{\frac{0.5 C_0}{N_1 C_1}}$  = coeficiente de fondo;  $D_0$

$\sqrt{\frac{B_0}{B_1}}$  = coeficiente de refracción;  $K_d$

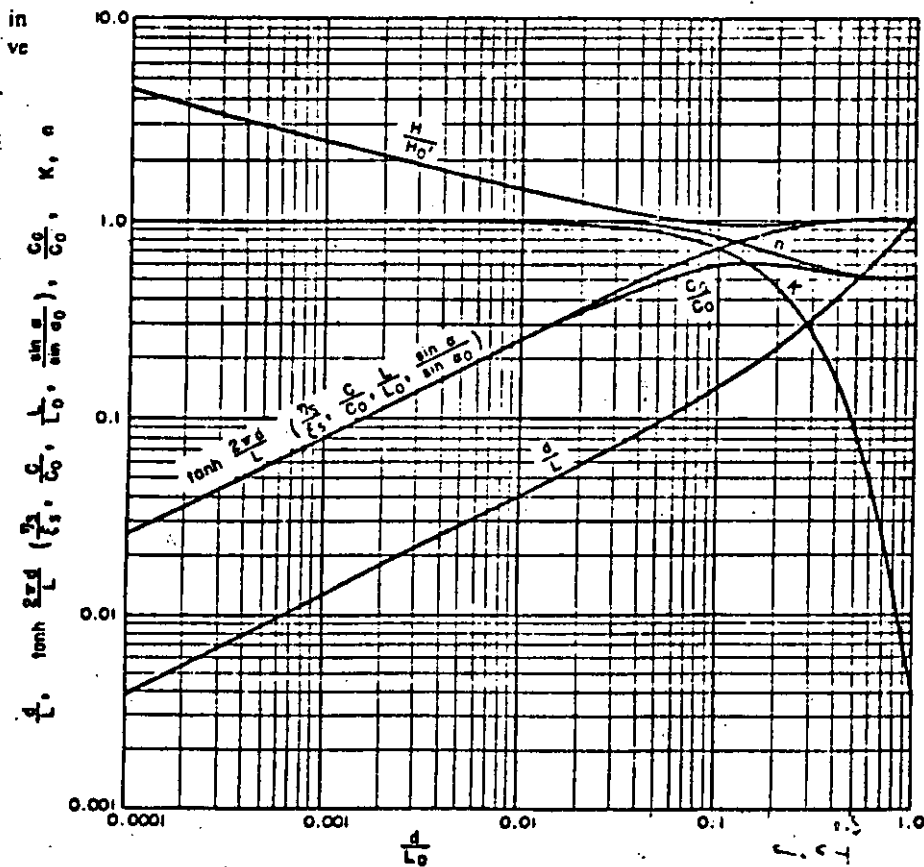
Cuando se da el caso en que  $B_1 = B_2$  (no hay cambio en la dirección)

$$H_2 = \sqrt{\frac{n_1 C_1}{n_2 C_2}} H_1 \quad H_0 = H_1 = \sqrt{\frac{n_2 C_2}{n_1 C_1}} H_0$$

En general se tienen dos tipos de fondo:

- curvas batimétricas irregulares
- curvas batimétricas rectas y paralelas

Análisis para el segundo caso



- H = wave height
- L = wave length
- C = wave phase velocity
- $\alpha$  = angle of wave crest with bottom
- d = water depth
- K = pressure response factor at bottom

- $C_0$  = wave group velocity
- n = ratio of wave group velocity to phase velocity
- 's' = superscript refers to waves not affected by refraction
- 'o' = subscript refers to deep water conditions
- $\gamma_s, \epsilon_s$  = vertical and horizontal components surface water particle displacements

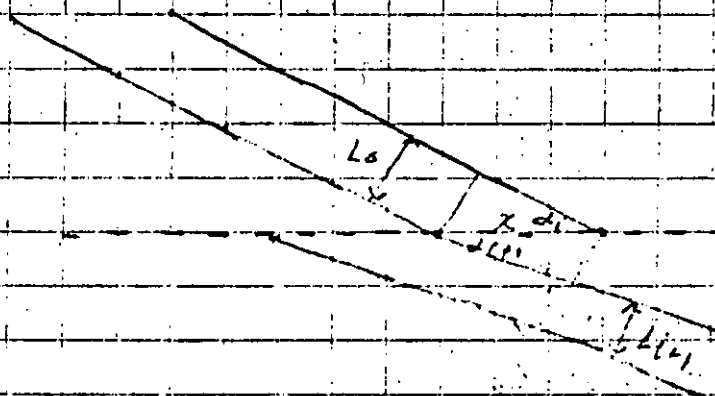


Las ondas se comienzan a refractar a partir de la profundidad  $d = 0.5 L_0$ .

$$H = H_0 \sqrt{\frac{C_0/C}{2n}} \sqrt{\frac{b_0}{b}}$$

$\sqrt{\frac{C_0/C}{2n}}$  es el coeficiente de fondo y se puede obtener del apéndice de Niegel

$\sqrt{\frac{b_0}{b}}$  es el coeficiente de refracción



$$\text{sen } \alpha_i = \frac{L_i}{x} = \frac{C_i}{x} \quad \text{sen } \alpha_{i+1} = \frac{L_{i+1}}{x} = \frac{L_{i+1}}{x}$$

$$\text{sen } \alpha_i = \frac{L_i}{x} = \frac{C_i}{x}$$

$$\text{sen } \alpha_{i+1} = \frac{L_{i+1}}{x} = \frac{C_{i+1}}{x}$$

$$\text{cos } \alpha_i = \frac{b_i}{x} \quad \text{cos } \alpha_{i+1} = \frac{b_{i+1}}{x}$$

$$\frac{\text{cos } \alpha_i}{\text{cos } \alpha_{i+1}} = \frac{b_i}{b_{i+1}}$$

$$\frac{\text{sen } \alpha_0}{\text{sen } \alpha_1} = \frac{c_0}{c_1} \quad ; \quad \text{sen } \alpha_1 = \frac{c_1}{c_0} \text{sen } \alpha_0$$

$$\frac{\text{sen } \alpha_1}{\text{sen } \alpha_2} = \frac{c_1}{c_2} \quad ; \quad \text{sen } \alpha_2 = \frac{c_2}{c_1} \text{sen } \alpha_1$$

$$\frac{\text{sen } \alpha_2}{\text{sen } \alpha_3} = \frac{c_2}{c_3} \quad ; \quad \text{sen } \alpha_3 = \frac{c_3}{c_2} \text{sen } \alpha_2$$

$$\frac{\text{sen } \alpha_3}{\text{sen } \alpha_4} = \frac{c_3}{c_4} \quad ; \quad \text{sen } \alpha_4 = \frac{c_4}{c_3} \text{sen } \alpha_3$$

$$\text{sen } \alpha_4 = \frac{c_4}{c_3} \frac{c_3}{c_2} \frac{c_2}{c_1} \frac{c_1}{c_0} \text{sen } \alpha_0$$

$$\therefore \text{sen } \alpha_4 = \frac{c_4}{c_0} \text{sen } \alpha_0$$

Esto es válido sólo para batimétricas rectas y paralelas

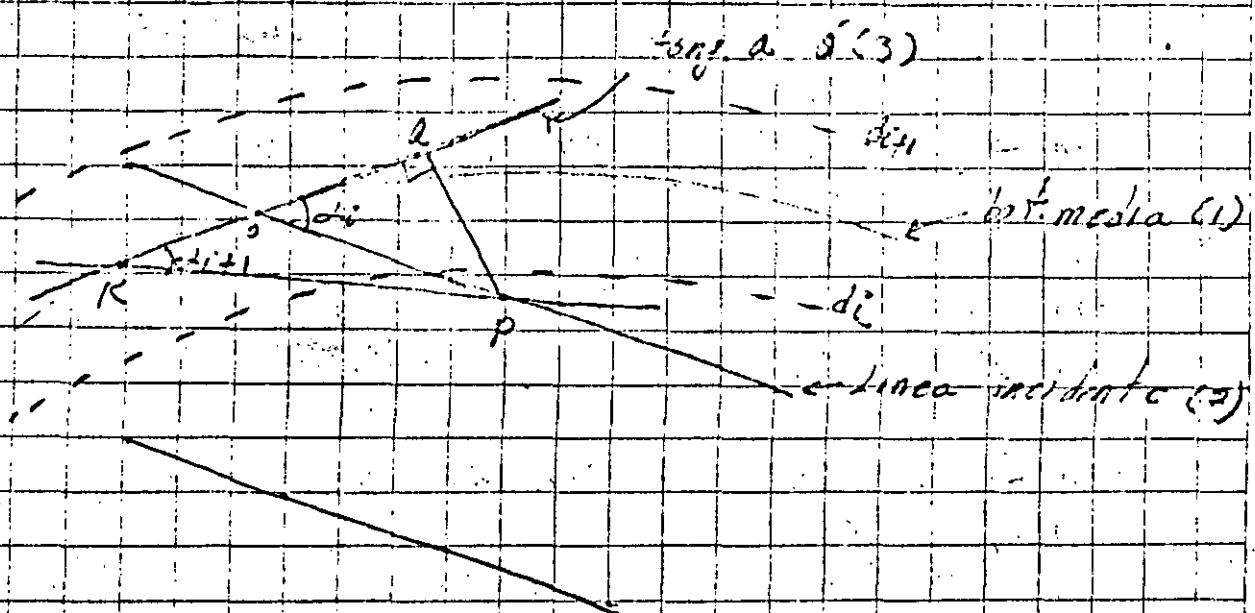
En general  $\text{sen } \alpha = \frac{c}{c_0} \text{sen } \alpha_0$

$$\alpha = \text{ang } \text{sen} \left( \frac{c}{c_0} \text{sen } \alpha_0 \right)$$

$$K_d = \sqrt{\frac{b_0'}{b}} = \sqrt{\frac{\cos \alpha_0}{\cos \alpha}}$$

(coeficiente de pende)  
refracción





$$\text{sen } \alpha_i = \frac{d_i}{OP} \quad \alpha_{i+1} = \arcsen \frac{L_i \text{ sen } \alpha_i}{L_i}$$

$$\text{sen } \alpha_{i+1} = \frac{d_i}{RP} \quad ; \quad \text{sen } \alpha_i = \frac{RP}{OP} = \frac{L_i}{L_{i+1}}$$

$$RP = \frac{L_i}{L_{i+1}} \quad \text{si } OP \text{ es unitaria}$$

Procedimiento

$L_i$	$d_i$	$d_i/L_0$	$d_i/L$	$L_0$	$L_i$	$L_i/L_{i+1}$
0	30	0,200	0,2251	133,27	13,60	1,125
1	20	0,133	0,1691	118,27	12,07	1,127
2	10	0,067	0,1114	90	9,18	1,314

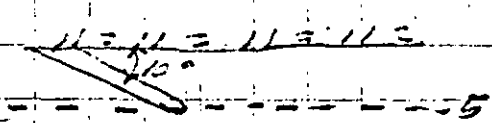
$$L_0 = 150 \text{ m} \quad L_0 = 15,3 \text{ m/seg} \quad T = 9,8 \text{ seg}$$

### Procedimiento:

1. se fija un punto O
2. se traza una tangente por O
3. se obtiene el frente el punto P
4. se obtiene el punto R

### Ejemplo:

En la batimétrica 5m existe una estructura que conforma con la línea de costa un ángulo de  $10^\circ$ . Interesa conocer la dirección y la altura de la ola sobre ella si en aguas profundas el oleaje tiene una longitud de  $100\text{m}$ ,  $H_0 = 5\text{m}$ ; y tiene una dirección de  $30^\circ$ . El fondo tiene una pendiente perpendicular a la línea de costa.



$$\frac{\sin \alpha}{\sin 2\alpha} = \frac{L}{L_0}$$

$$\frac{\sin \alpha}{\sin 2\alpha} = 0.5 \quad \frac{\sin \alpha}{\sin 2\alpha} = \frac{L}{L_0}$$

$$\alpha = \arcsin \left( \sin 2\alpha \cdot \frac{L}{L_0} \right)$$

$$k_d = \sqrt{\frac{\cos 2\alpha}{\cos \alpha}} \quad L_0 = \sqrt{\frac{g L_0^3}{2\pi}} = \sqrt{\frac{9.81 (100)^3}{2\pi}} = 12.5 \text{ m/sec}$$

$$T = \frac{L_0}{C} = \frac{100}{12.5} = 8.1$$

$$C(5\text{m}) = ? \quad \frac{d}{L_0} = \frac{5}{100} = 0.05 \rightarrow \frac{d}{L} = 0.094/6$$

$$L = 53.10 \text{ m} ; c = \frac{L}{T} = \frac{53.10}{8} = 6.64 \text{ m/s}$$

$$\alpha = \arcsin \left( \frac{6.64 \sin 30}{12.5} \right) = 15.18^\circ$$

$$K_d = \frac{\cos 30^\circ}{\cos 15.18^\circ} = 0.9472$$

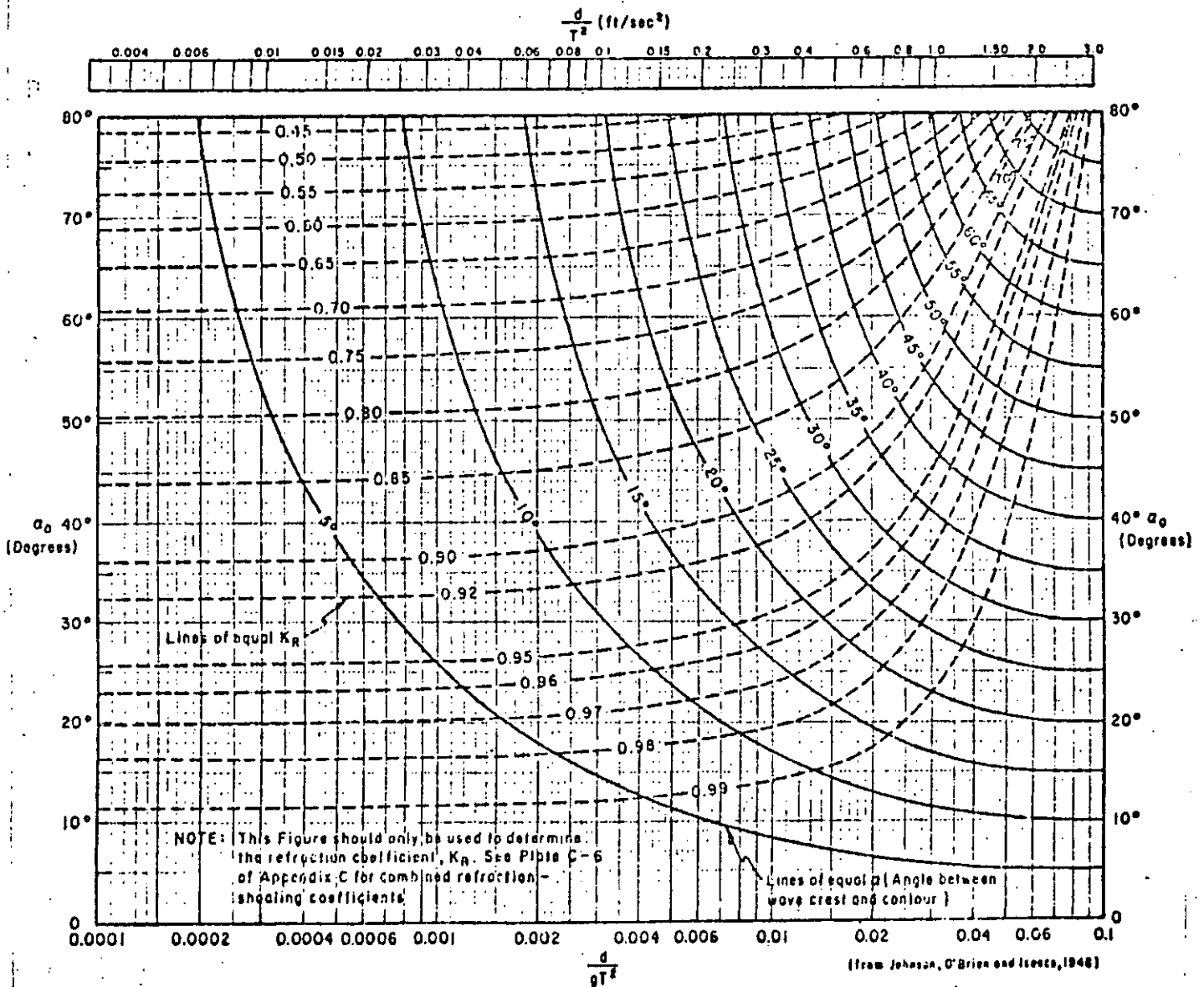


Figure 2-19. Changes in Wave Direction and Height Due to Refraction on Slopes with Straight, Parallel Depth Contours

# Hidráulica Marítima y de Estuarios

## Ejemplo 2.

Cálculo de la altura de ola,  $H$  para los puntos A, B y C de la fig. 1

Datos:  $T_o = 7 \text{ seg}$ ,  $H_o = 6 \text{ m}$

$$L_o = \frac{gT^2}{2\pi} = \frac{9.81(7)^2}{2\pi} = 76.5 \text{ m}$$

$$\frac{d_o}{L_o} = 0.5 \quad ; \quad d_o = 0.5 L_o = 0.5(76.5) = 38.25 \text{ m}$$

$$C_o = \frac{L_o}{T_o} = \frac{76.5}{7} = 10.93 \text{ m/seg.}$$

i	$d_i$	$d_i/L_o$	$d_i/L_i$	$L_i$	$C_i$	$C_i/C_o$
1	35	0.4575	0.4604	76.02	10.86	1.006
2	30	0.3921	0.3975	75.47	10.78	1.007
3	25	0.3268	0.3364	74.32	10.62	1.015
4	20	0.2614	0.2780	71.94	10.28	1.033
5	15	0.1961	0.2219	67.60	9.66	1.064
6	10	0.1307	0.1672	59.81	8.54	1.131
7	5	0.0654	0.1096	45.62	6.52	1.310

Cálculo del coeficiente de fondo  $D_d$

$$d = 2.5 \text{ m} \quad ; \quad \frac{d}{L_o} = \frac{2.5}{76.5} = 0.033 \Rightarrow \frac{d}{L} = 0.07507$$

$$L = 33.30 \text{ m} \quad \text{y} \quad D_d = 1.104 \quad (\text{tablas de Wiegell})$$

Cálculo de  $H$  en A

$$b_o = 1 \text{ cm} \quad ; \quad b = 4.8 \text{ cm} \quad ; \quad K_d = \sqrt{b_o/b} = \sqrt{1/4.8} = 0.456$$

$$H_A = D_d K_d H = (1.104)(0.456)(6)$$

$$\therefore H_A = 3.02 \text{ m}$$

- Cálculo de H en B

$$b_0 = 4.7 \text{ cm}, b = 1.7 \text{ cm}; K_d = \sqrt{b_0/b} = \sqrt{4.7/1.7} = 1.663$$

$$H_B = H D_d K_d = 6 (1.104) (1.663)$$

$$\therefore H_B \doteq 11.01 \text{ m}$$

- Cálculo de H en C

$$b_0 = 1 \text{ cm}; b = 0.9 \text{ cm}; K_d = \sqrt{b_0/b} = \sqrt{1/0.9} = 1.054$$

$$H_C = H D_d K_d = 6 (1.104) (1.054)$$

$$\therefore H_C = 6.98 \text{ m}$$

= Procedimiento alternativa para calcular H<sub>c</sub>

de la figura  $\alpha_0 = 12^\circ$        $D_d = 1.104$

$$C_0 = 10.93 \text{ m/seg}; L_{2.5} = 33.30; C_{2.5} = \frac{33.3}{7} = 4.757 \text{ m/seg}$$

$$\alpha = \arcsin \left( \sin \alpha_0 \frac{C}{C_0} \right) = \arcsin \left( \sin 12^\circ \cdot \frac{4.757}{10.93} \right) = 5.192^\circ$$

$$K_d = \sqrt{\frac{\cos \alpha_0}{\cos \alpha}} = \sqrt{\frac{\cos 12^\circ}{\cos 5.192^\circ}} = 0.991$$

$$H_C = H D_d K_d = 6 (1.104) (0.991)$$

$$H_C \doteq 6.57 \text{ m}$$

$$\frac{d}{2T^2} = \frac{2.5}{9.81(7)^2}$$

Anotaciones en metros  
Escala: 1 cm = 200 m

ENCONTRAR LA DIRECCION DEL OLEAJE Y SU ALTURA EN LOS PUNTOS A, B Y C. Si en aguas profundas el oleaje tiene un periodo de 7 s, altura de 6 m y la dirección mostrada en esta figura.  
(Para el punto C se puede considerar botinas rectas y paralelas)

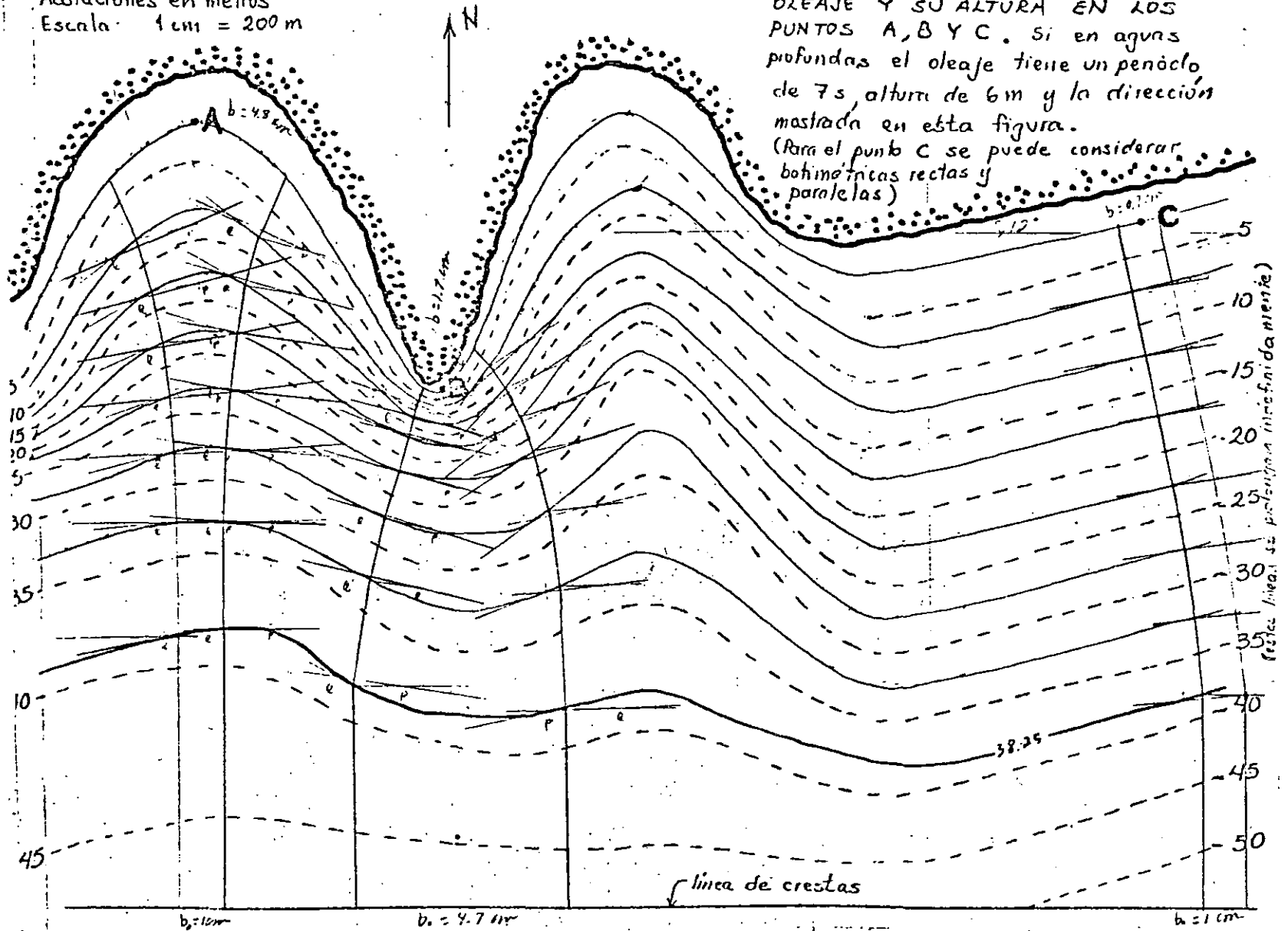


Fig 1

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## 2.3 Difracción del oleaje

### Hipótesis de partida

- 1- barreras semiinfinitas
- 2- oleaje lineal
- 3- profundidad constante

$$\frac{\partial^2 \phi}{\partial x^2} + \frac{\partial^2 \phi}{\partial y^2} + \frac{\partial^2 \phi}{\partial z^2} = 0 \quad (1)$$

$$\frac{\partial \phi}{\partial z} \Big|_{z=-d} = 0 \quad (2) \quad \eta = -\frac{1}{g} \frac{\partial \phi}{\partial t} \Big|_{z=0} \quad (3)$$

$$\phi = \frac{a \sqrt{g}}{k} \frac{\cosh k(z+d)}{\sinh h k d} \sin(kx - \sqrt{g} t)$$

se propone  $\phi = A f(x, y) \cosh h k(z+d) e^{i\sqrt{g} t}$

$$e^{i\sqrt{g} t} = \cos \sqrt{g} t + i \sin \sqrt{g} t$$

$f(x, y)$  es una función compleja

$A$  es una constante de proporc. de la amplitud

$$\frac{\partial \phi}{\partial x} = A \cosh h k(z+d) e^{i\sqrt{g} t} \frac{\partial f}{\partial x}$$

$$\frac{\partial^2 \phi}{\partial x^2} = A \cosh h k(z+d) e^{i\sqrt{g} t} \frac{\partial^2 f}{\partial x^2}$$

$$\frac{\partial^2 \phi}{\partial y^2} = A \cosh h k(z+d) e^{i\sqrt{g} t} \frac{\partial^2 f}{\partial y^2}$$

$$\frac{\partial \phi}{\partial z} = A f(x, y) e^{iVt} k \sinh k(z+d)$$

$$\frac{\partial^2 \phi}{\partial z^2} = A f(x, y) e^{iVt} k^2 \cosh k(z+d)$$

subst. en 1

$$A \cosh k(z+d) e^{iVt} \frac{\partial^2 f}{\partial x^2} + A \cosh k(z+d) e^{iVt} \frac{\partial^2 f}{\partial y^2} +$$

$$+ A \cosh k(z+d) e^{iVt} k^2 f = 0$$

$$\boxed{\frac{\partial^2 f}{\partial x^2} + \frac{\partial^2 f}{\partial y^2} + k^2 f = 0} \quad \text{Ec. de Helmholtz (4)}$$

$$\text{de 3} \quad \eta = -\frac{1}{g} A f \cosh k(z+d) iV e^{iVt} \Big|_{z=0}$$

$$\boxed{\eta = -\frac{AV f i \cosh kd e^{iVt}}{g}}$$

oleaje incidente (propagación en la dirección y)

$$f = e^{iky} \quad (6)$$

$$\eta_i = -\frac{AV i \cosh kd e^{i(ky+Vt)}}{g} \quad (7)$$

al dividir (7) entre (6)

$$\frac{\eta}{\eta_i} = f e^{iky}$$

$$\frac{H}{H_i} = |f| = K \text{ o } \rho$$



Table 8.1. DIFFRACTION COEFFICIENTS,  $K'$ , AS A FUNCTION OF INCIDENT WAVE ANGLE,  $\theta_0$ , AND POSITION,  $r/L$  AND  $\theta$ , (from Wiegel, 1962)

Value of $r/L$	Value of $\theta$ (degrees)												
	0	15	30	45	60	75	90	105	120	135	150	165	180
$\theta_0 = 15^\circ$													
1/2	0.49	0.77	0.82	0.90	0.97	1.01	1.03	1.02	1.01	0.99	0.99	1.00	1.00
1	0.38	0.73	0.83	0.95	1.04	1.04	0.99	0.98	1.01	1.01	1.00	1.00	1.00
2	0.21	0.68	0.86	1.05	1.03	0.97	1.02	0.99	1.00	1.00	1.00	1.00	1.00
5	0.13	0.63	0.99	1.04	1.03	1.02	0.99	0.99	1.00	1.01	1.00	1.00	1.00
10	0.35	0.58	1.10	1.05	0.92	0.99	1.01	1.00	1.00	1.00	1.00	1.00	1.00
$\theta_0 = 30^\circ$													
1/2	0.61	0.63	0.68	0.76	0.87	0.97	1.03	1.05	1.03	1.01	0.99	0.95	1.00
1	0.50	0.53	0.63	0.78	0.95	1.06	1.05	0.98	0.98	1.01	1.01	0.97	1.00
2	0.40	0.44	0.59	0.84	1.07	1.03	0.96	1.02	0.98	1.01	0.99	0.95	1.00
5	0.27	0.32	0.55	1.00	1.04	1.04	1.02	0.99	0.99	1.00	1.01	0.97	1.00
10	0.20	0.24	0.54	1.12	1.06	0.97	0.99	1.01	1.00	1.00	1.00	0.98	1.00
$\theta_0 = 45^\circ$													
1/2	0.49	0.50	0.55	0.63	0.73	0.85	0.96	1.04	1.06	1.04	1.00	0.99	1.00
1	0.38	0.40	0.47	0.59	0.76	0.95	1.07	1.06	0.98	0.97	1.01	1.01	1.00
2	0.29	0.31	0.39	0.56	0.83	1.08	1.04	0.96	1.03	0.98	1.01	1.00	1.00
5	0.18	0.20	0.29	0.54	1.01	1.04	1.05	1.03	1.00	0.99	1.01	1.00	1.00
10	0.13	0.15	0.22	0.53	1.13	1.07	0.96	0.98	1.02	0.99	1.00	1.00	1.00
$\theta_0 = 60^\circ$													
1/2	0.40	0.41	0.45	0.52	0.60	0.72	0.85	1.13	1.04	1.06	1.03	1.01	1.00
1	0.31	0.32	0.36	0.44	0.57	0.75	0.96	1.08	1.06	0.98	0.98	1.01	1.00
2	0.22	0.23	0.28	0.37	0.55	0.83	1.08	1.04	0.95	1.03	0.98	1.01	1.00
5	0.14	0.15	0.19	0.28	0.53	1.01	1.04	1.05	1.03	0.99	0.98	1.00	1.00
10	0.10	0.11	0.13	0.21	0.52	1.14	1.07	0.96	0.98	1.01	1.00	1.00	1.00
$\theta_0 = 75^\circ$													
1/2	0.34	0.35	0.38	0.42	0.50	0.59	0.71	0.85	0.97	1.04	1.05	1.02	1.00
1	0.25	0.26	0.29	0.34	0.43	0.56	0.75	0.95	1.02	1.06	0.92	0.92	1.00
2	0.18	0.19	0.22	0.26	0.36	0.54	0.83	1.09	1.04	0.96	1.03	0.99	1.00
5	0.12	0.12	0.13	0.17	0.27	0.52	1.01	1.04	1.05	1.03	0.99	0.99	1.00
10	0.08	0.08	0.10	0.13	0.20	0.52	1.14	1.07	0.95	0.98	1.01	1.00	1.00
$\theta_0 = 90^\circ$													
1/2	0.31	0.31	0.33	0.36	0.41	0.49	0.59	0.71	0.85	0.96	1.03	1.03	1.00
1	0.22	0.23	0.24	0.28	0.33	0.42	0.56	0.75	0.96	1.07	1.05	0.99	1.00
2	0.16	0.16	0.18	0.20	0.25	0.35	0.54	0.69	1.08	1.04	0.96	1.02	1.00
5	0.10	0.10	0.11	0.13	0.16	0.27	0.53	1.01	1.04	1.05	1.02	0.99	1.00
10	0.07	0.07	0.08	0.09	0.13	0.20	0.52	1.14	1.07	0.96	0.99	1.01	1.00
$\theta_0 = 105^\circ$													
1/2	0.28	0.28	0.29	0.32	0.35	0.41	0.49	0.59	0.72	0.85	0.97	1.01	1.00
1	0.20	0.20	0.24	0.23	0.27	0.33	0.42	0.56	0.75	0.95	1.06	1.04	1.00
2	0.14	0.14	0.13	0.17	0.20	0.25	0.35	0.54	0.83	1.08	1.03	0.97	1.00
5	0.09	0.09	0.10	0.11	0.13	0.17	0.27	0.52	1.02	1.04	1.04	1.02	1.00
10	0.07	0.06	0.08	0.08	0.09	0.12	0.20	0.52	1.14	1.07	0.97	0.99	1.00
$\theta_0 = 120^\circ$													
1/2	0.25	0.26	0.27	0.28	0.31	0.35	0.41	0.50	0.60	0.73	0.87	0.97	1.00
1	0.18	0.19	0.19	0.21	0.23	0.27	0.33	0.43	0.57	0.76	0.95	1.04	1.00
2	0.13	0.13	0.14	0.14	0.17	0.20	0.26	0.36	0.55	0.83	1.07	1.03	1.00
5	0.08	0.08	0.08	0.09	0.11	0.13	0.16	0.27	0.53	1.01	1.04	1.03	1.00
10	0.06	0.06	0.06	0.07	0.07	0.09	0.13	0.20	0.52	1.13	1.06	0.98	1.00
$\theta_0 = 135^\circ$													
1/2	0.24	0.24	0.25	0.26	0.28	0.32	0.36	0.42	0.52	0.63	0.76	0.90	1.00
1	0.18	0.17	0.18	0.19	0.21	0.23	0.28	0.34	0.44	0.59	0.78	0.95	1.00
2	0.12	0.12	0.13	0.14	0.14	0.17	0.20	0.26	0.37	0.56	0.84	1.05	1.00
5	0.08	0.07	0.08	0.08	0.09	0.11	0.13	0.17	0.28	0.54	1.00	1.04	1.00
10	0.05	0.06	0.06	0.06	0.07	0.08	0.09	0.13	0.21	0.53	1.12	1.05	1.00
$\theta_0 = 150^\circ$													
1/2	0.23	0.23	0.24	0.25	0.27	0.29	0.33	0.38	0.45	0.54	0.68	0.83	1.00
1	0.16	0.17	0.17	0.18	0.19	0.22	0.24	0.29	0.36	0.47	0.63	0.83	1.00
2	0.12	0.12	0.12	0.13	0.14	0.15	0.18	0.22	0.28	0.39	0.59	0.86	1.00
5	0.07	0.07	0.08	0.08	0.08	0.10	0.11	0.13	0.18	0.29	0.55	0.99	1.00
10	0.05	0.05	0.05	0.06	0.06	0.07	0.08	0.10	0.13	0.22	0.54	1.10	1.00

Table 8.1. (cont.)

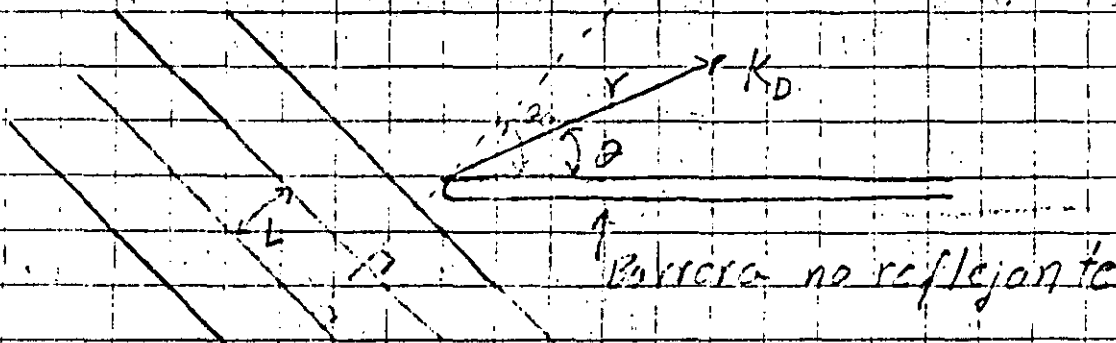
$\theta_0 = 165^\circ$

1/2	0.23	0.23	0.23	0.24	0.26	0.28	0.31	0.35	0.41	0.50	0.63	0.79	1.00
1	0.16	0.16	0.17	0.17	0.19	0.20	0.23	0.26	0.32	0.40	0.53	0.73	1.00
2	0.11	0.11	0.12	0.12	0.13	0.14	0.16	0.19	0.23	0.31	0.44	0.68	1.00
5	0.07	0.07	0.07	0.07	0.08	0.09	0.10	0.12	0.15	0.20	0.32	0.63	1.00
10	0.05	0.05	0.05	0.06	0.06	0.06	0.07	0.08	0.11	0.11	0.21	0.58	1.00

$\theta_0 = 180^\circ$

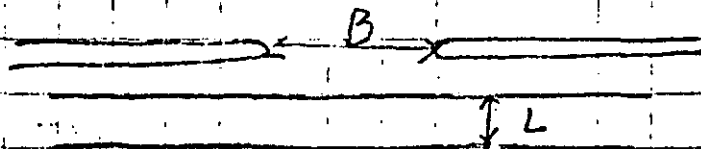
1/2	0.20	0.25	0.23	0.24	0.25	0.28	0.31	0.34	0.40	0.49	0.61	0.78	1.00
1	0.10	0.17	0.16	0.18	0.18	0.23	0.22	0.25	0.31	0.38	0.50	0.70	1.00
2	0.02	0.09	0.12	0.12	0.13	0.18	0.16	0.18	0.22	0.29	0.40	0.60	1.00
5	0.02	0.06	0.07	0.07	0.07	0.08	0.10	0.12	0.14	0.18	0.27	0.46	1.00
10	0.01	0.05	0.05	0.04	0.06	0.07	0.07	0.08	0.10	0.13	0.20	0.36	1.00

Difracción para una barrera semi-infinita.

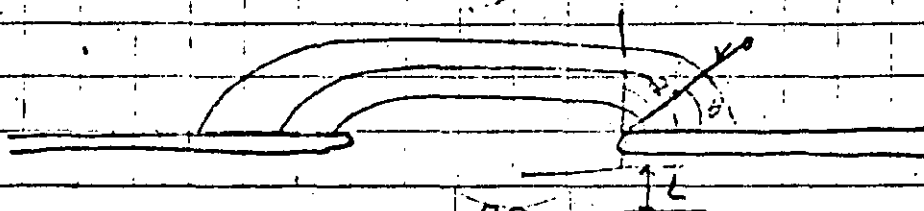


Se entra a los renglones correspondientes de (la tabla 8.1)  $\theta_0$ , luego con la relación  $r/L$  y  $\theta$  se encuentra  $K_D$

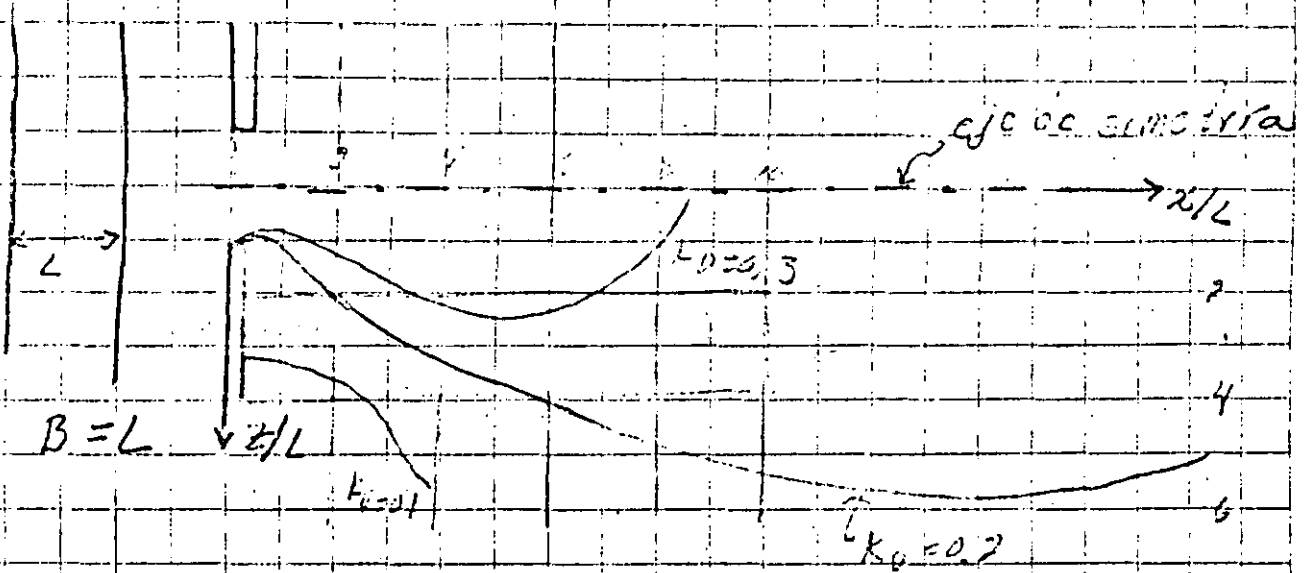
Difracción para una abertura en una barrera



El caso si  $B > 5L$  se analiza por tramos separados

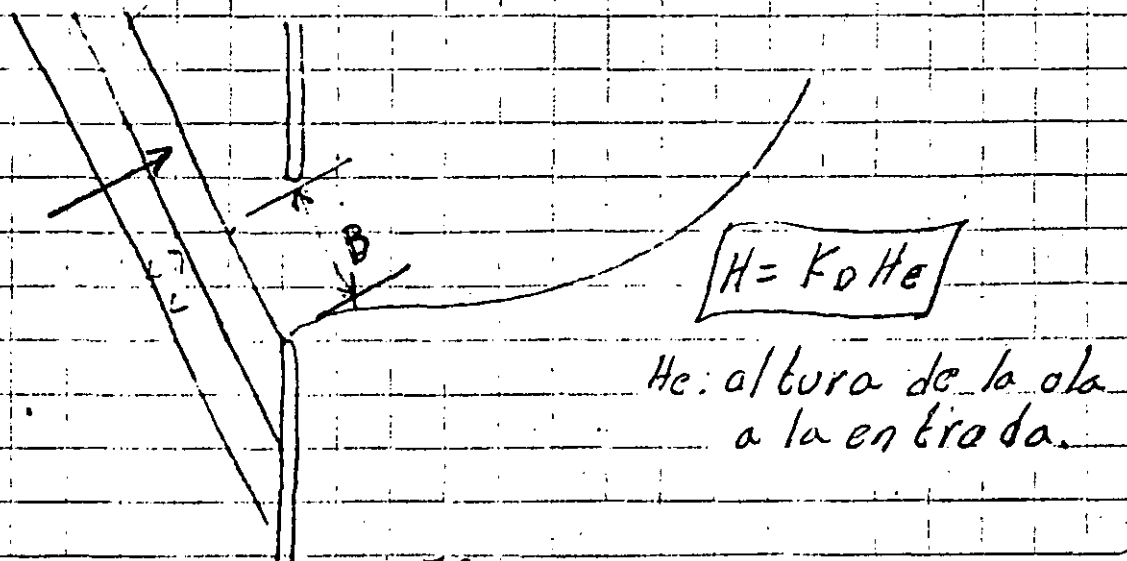


B) sí  $B \leq 5L$



En todos los casos anteriores, los frentes de onda son paralelos a el ancho.

Para el caso en que el frente de onda no sea paralelo al ancho se hace la siguiente consideración.



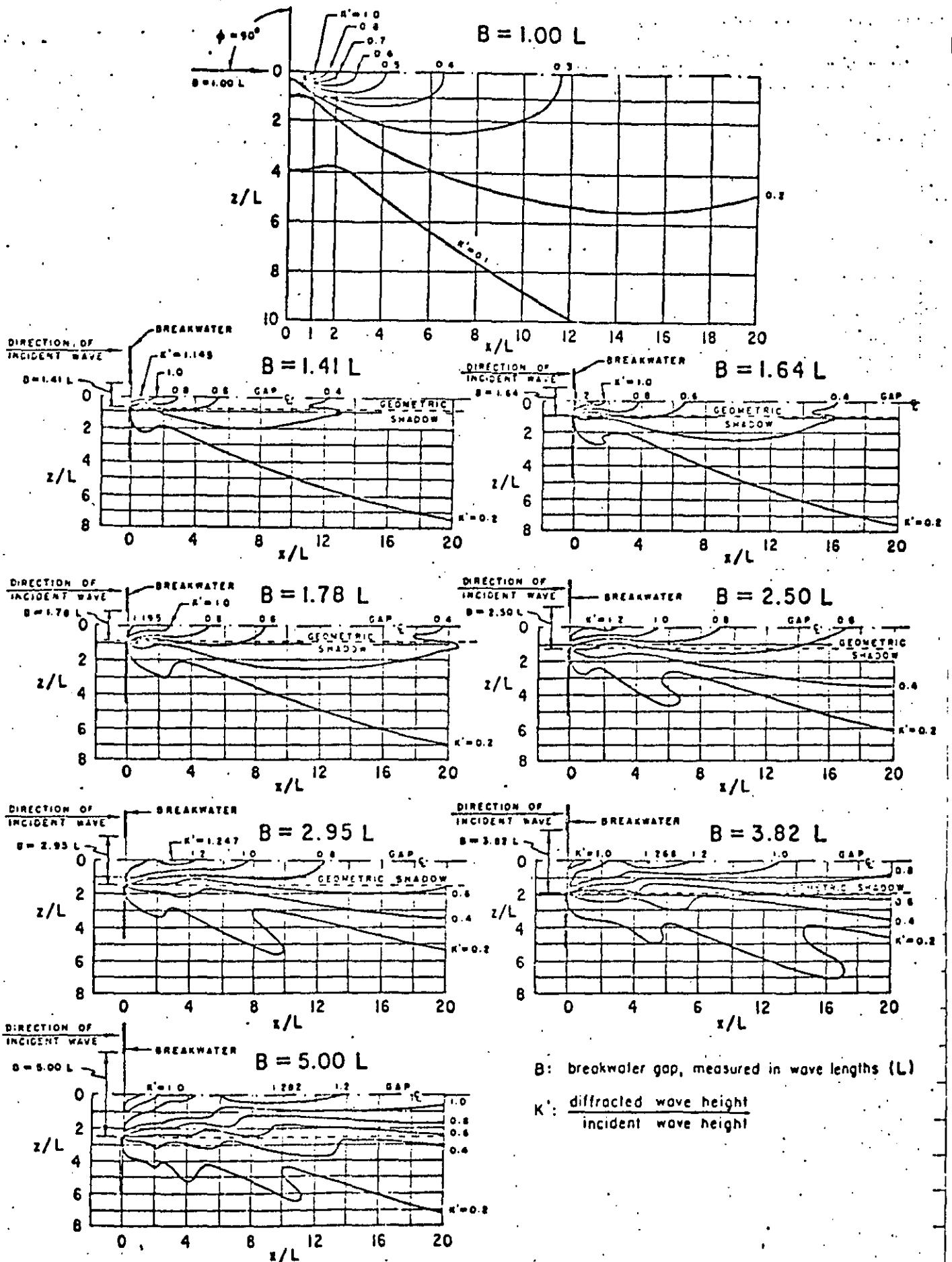
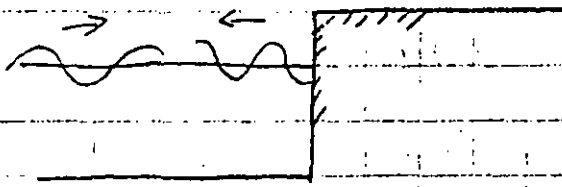


Fig. 8.12. Diffraction of waves at breakwater gap—contours of equal diffraction coefficient (after Johnson, 1952)

## 2.4 Reflexión

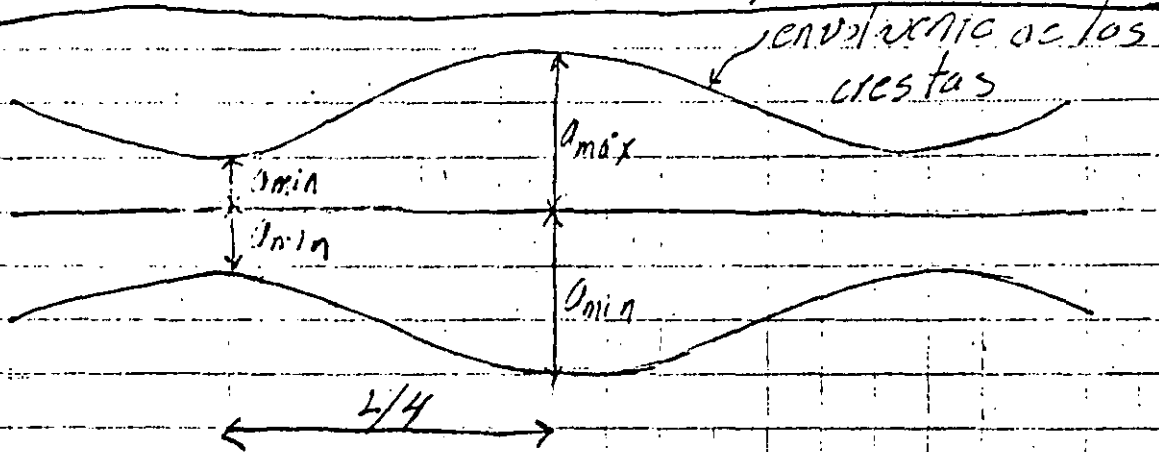
Cuando una onda choca con un obstáculo se puede presentar otra onda que parta desde este a este fenómeno corresponde a la reflexión.

$$\eta_r = a_r \cos(kx + \tau t) \quad \eta_i = a_i \cos(kx + \tau t)$$



POR UNA IDENTIDAD DE LAS FUNCIONES CIRCULARES:  
 $\eta = a_i \cos(kx + \tau t) + a_r \cos(kx + \tau t)$

$$\eta = (a_i + a_r) \cos kx \cos \tau t + (a_i - a_r) \sin kx \sin \tau t$$



$$A_{\max} = a_i + a_r \quad ; \quad A_{\min} = a_i = a_r$$

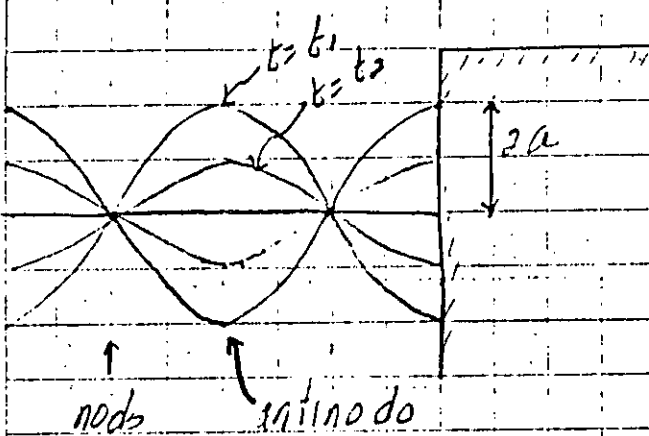
de aquí  $a_i = \frac{A_{\max} + A_{\min}}{2}$        $a_r = \frac{A_{\max} - A_{\min}}{2}$

Coefficiente de reflexión  $K_R = \frac{a_r}{a_i} = \frac{A_{\max} - A_{\min}}{A_{\max} + A_{\min}}$

Lo anterior corresponde a una reflexión parcial y se tiene  $a_{\text{min}} \neq 0$

La reflexión total ocurre cuando  $a_i = a_r = a$

$$n = 2a \cos kx \cos Tt$$



La función potencial  $\phi$  es

$$\phi = \phi_i + \phi_r \quad a_i = a_r$$

$$\phi = \frac{a_i \sqrt{g}}{k} \frac{\cosh k(z+d)}{\sinh kd} \sin(kx - Tt)$$

$$+ \frac{a_r \sqrt{g}}{k} \frac{\cosh k(z+d)}{\sinh kd} \sin(kx + Tt)$$

ó sea  $\phi = \frac{2a \sqrt{g}}{k} \frac{\cosh k(z+d)}{\sinh kd} \cos kx \cos Tt$

$$u = \frac{\partial \phi}{\partial x}$$

coeficientes de reflexión

tipos de estructura	$K_L$
Muros verticales (o si verticales)	0.8 - 1
Estructuras taludes 2 y 3	0.4 - 0.8
Estructuras de bloques de concreto	0.2 - 0.4
Terra plen	0.3 - 0.5
playas	0.1 - 0.2

## Reflexión del oleaje en playas.

$$K_L = \alpha_1 \alpha_2$$

$\alpha_1$  depende de la rugosidad y permeabilidad de la playa.

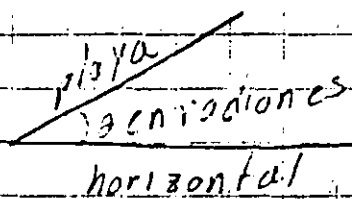
0.8 para playas lisas impermeables

0.3 - 0.6 para playas rugosas impermeables

$\alpha_2$  depende de la pendiente de la playa y esbeltez  $(H_0/L_0)$  del oleaje en aguas profundas.

Para evaluar  $\alpha_2$  obtengase primero

$$\left(\frac{H_0}{L_0}\right)_{\max} = \left(\frac{2\theta}{\pi}\right)^{1/2} \frac{\sin^2 \theta}{\pi}$$

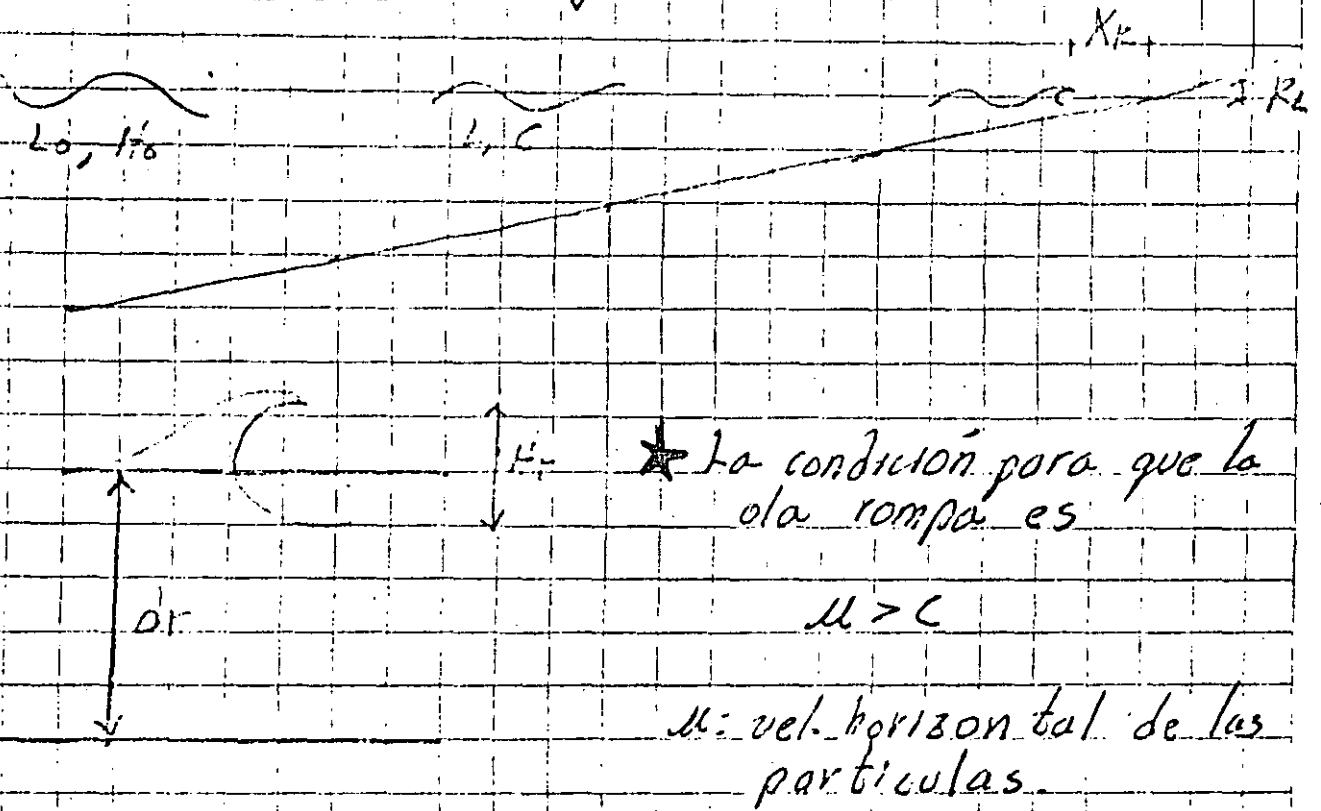


$$\alpha_2 = \frac{(H_0/L_0)_{\max}}{(H_0/L_0)}$$

sólo si se tiene un valor menor de 1

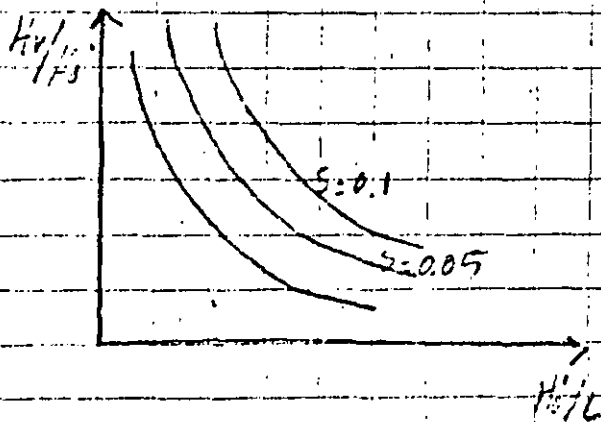
en caso contrario, dese a  $\alpha_2$  el valor de 1.

## 2.5 Rompiente del oleaje



la forma en que rompe la ola se clasifica en los siguientes tipos:  
 continua, rodante y ondulante

Criterio de Goda



$H_{ref} = H_0 D_d K_d$

$D_d = H/H_0' \Rightarrow H = H_0' \frac{H}{H_0'} K_d$

$H_0' = H_0 K_d$



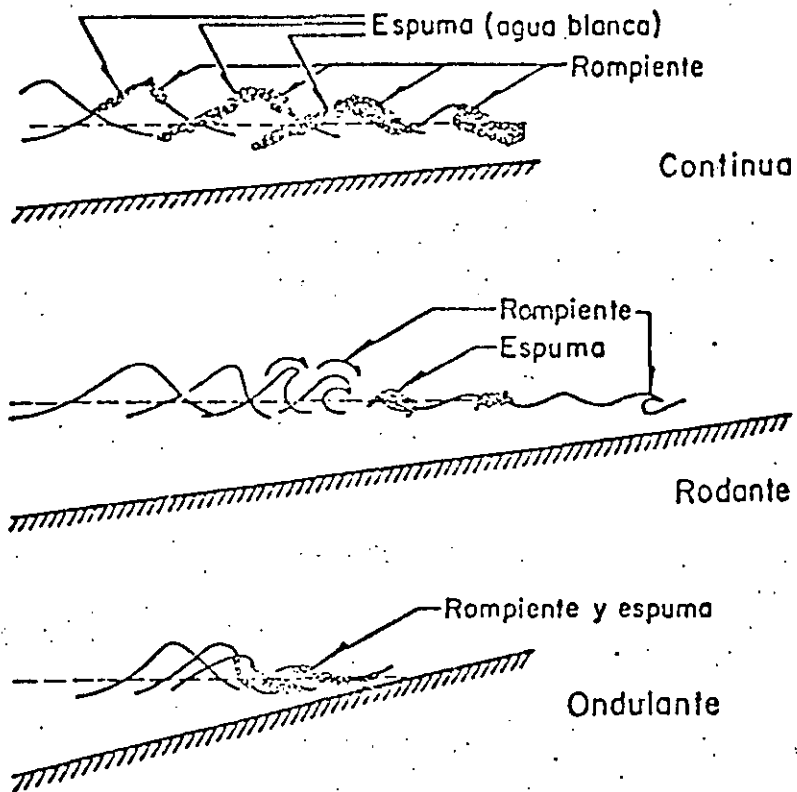


FIG. 1. Clasificación del oleaje rompiente, según Sorensen

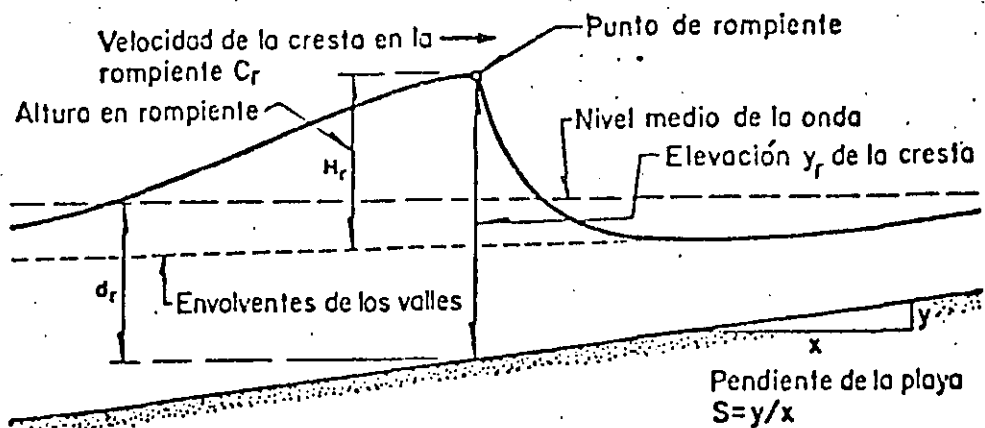


FIG. 2. Términos utilizados para definir a la ola rompiente, según Iversen ( 1952 )

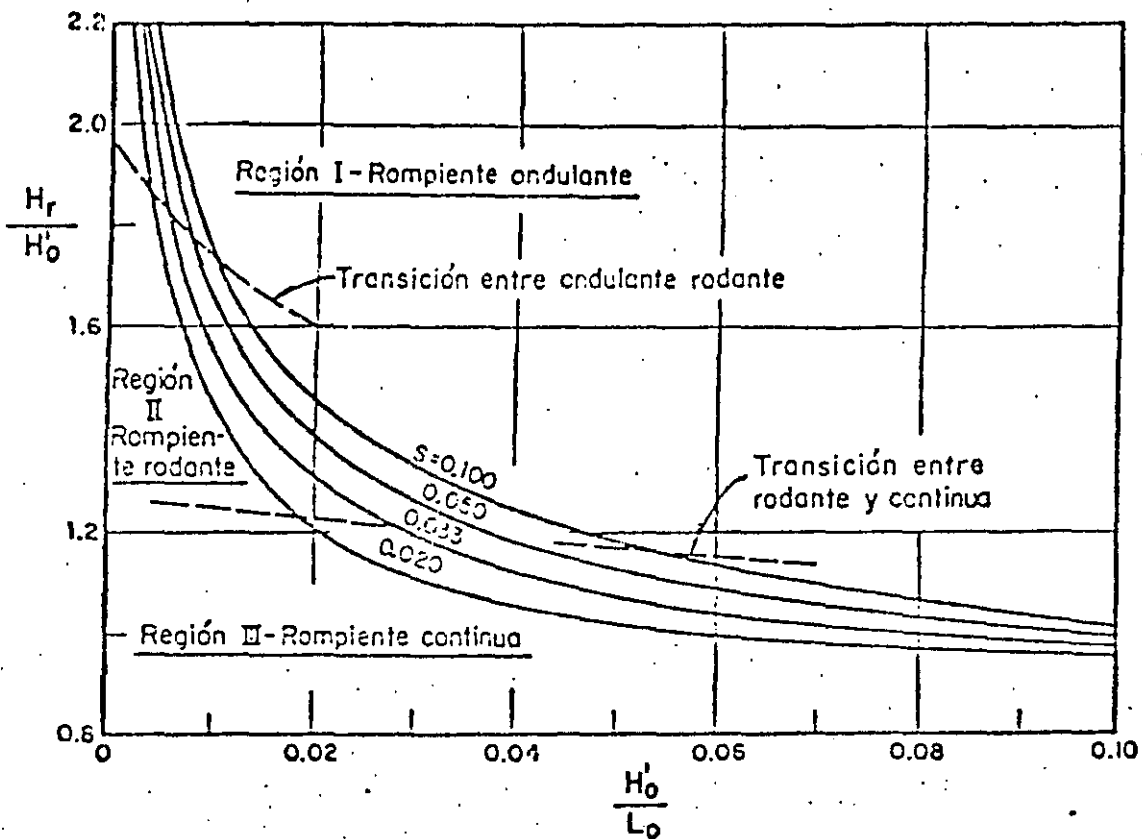


FIG. 11.3 Altura de la ola rompiente y su clasificación en función de la pendiente de la playa, altura de la ola en aguas profundas y esbeltez de la ola, según el CERC ( ref 6 )

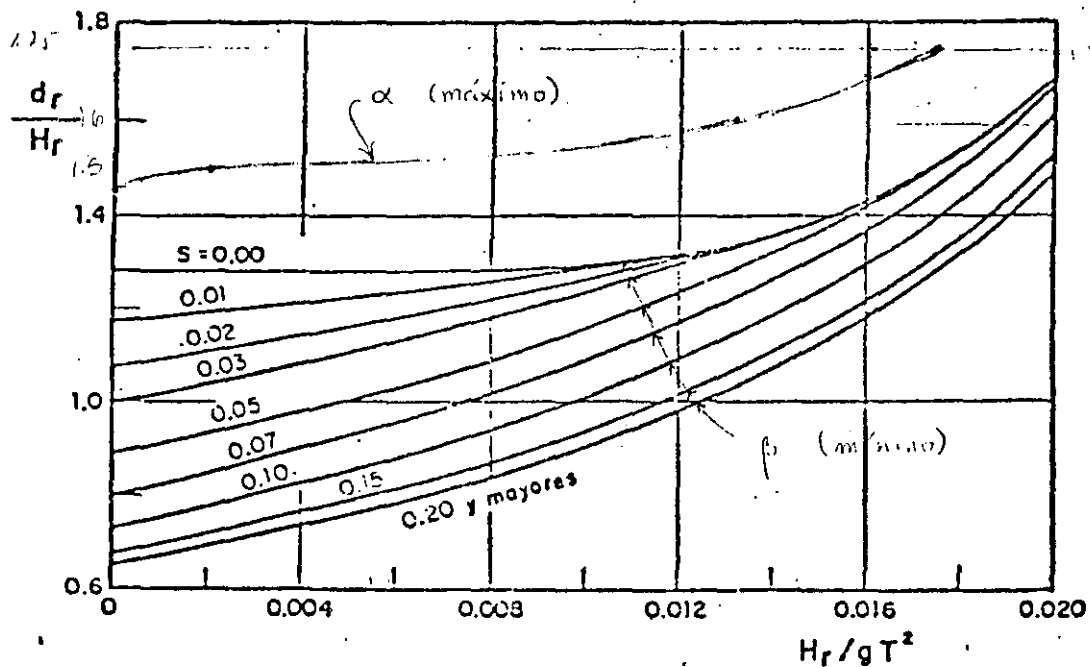
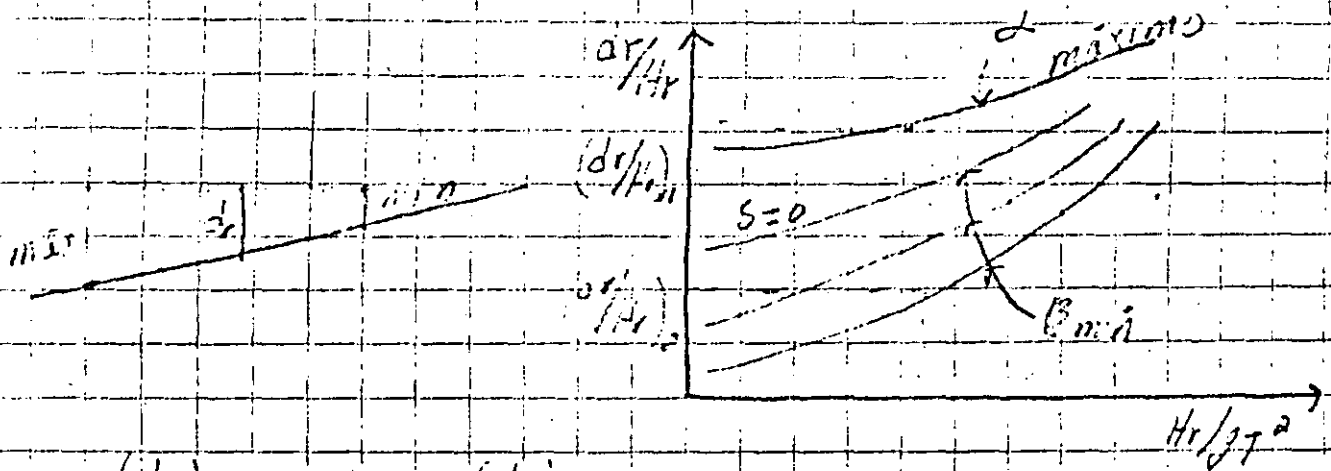


FIG. 11.4 Profundidad en rompiente, en función de la pendiente de la playa, altura de la ola rompiente y periodo de la ola, según el CERC ( ref 6 )

El coeficiente de fondo es  $K_d=1$  cuando el frente de onda es paralelo a la batimétrica.

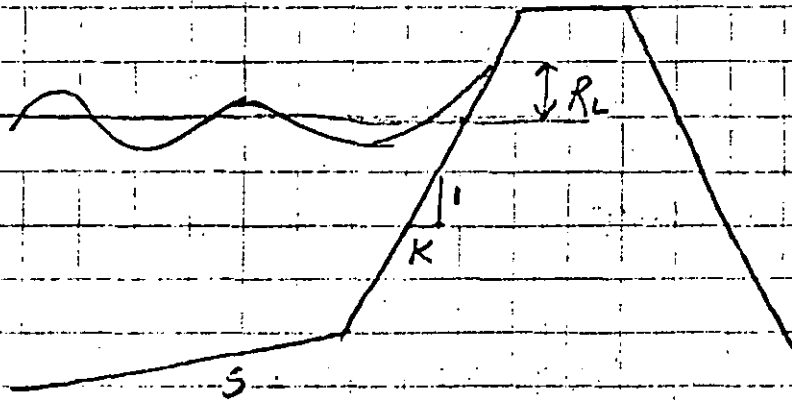


$$\left(\frac{dr}{Hr}\right)_2 \leq \frac{dr}{Hr} \leq \left(\frac{dr}{Hr}\right)_1$$

Para el oleaje en aguas profundas

$$\frac{H_e}{L_e} \leq 0.147 \doteq \frac{1}{7} \quad (\text{rompiente en aguas profundas})$$

## ★ Alcance de la ola (run up)



1. La ola rompe antes de llegar a la estructura
2. La ola no rompe antes de llegar a la estructura
  - 2.1 La ola rompe sobre la estructura
  - 2.2 La ola no rompe sobre la estructura.

2.-

$$R_r = H_o D_d F \left[ \sqrt{\frac{\pi}{2\alpha} + \frac{\pi H}{L} \cot \alpha} \right]$$

$\alpha$  en radianes =  $\text{ang } \tan^{-1} k$

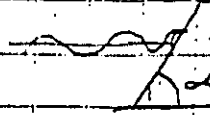
## Alcance de la ola (Run-up)

①



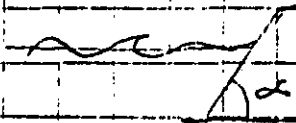
no rompe

②



la ola rompe sobre la estructura

③



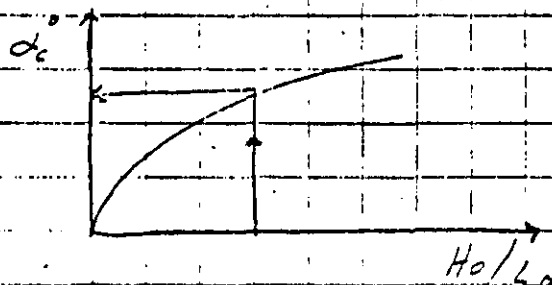
la ola rompe antes de llegar a la estructura

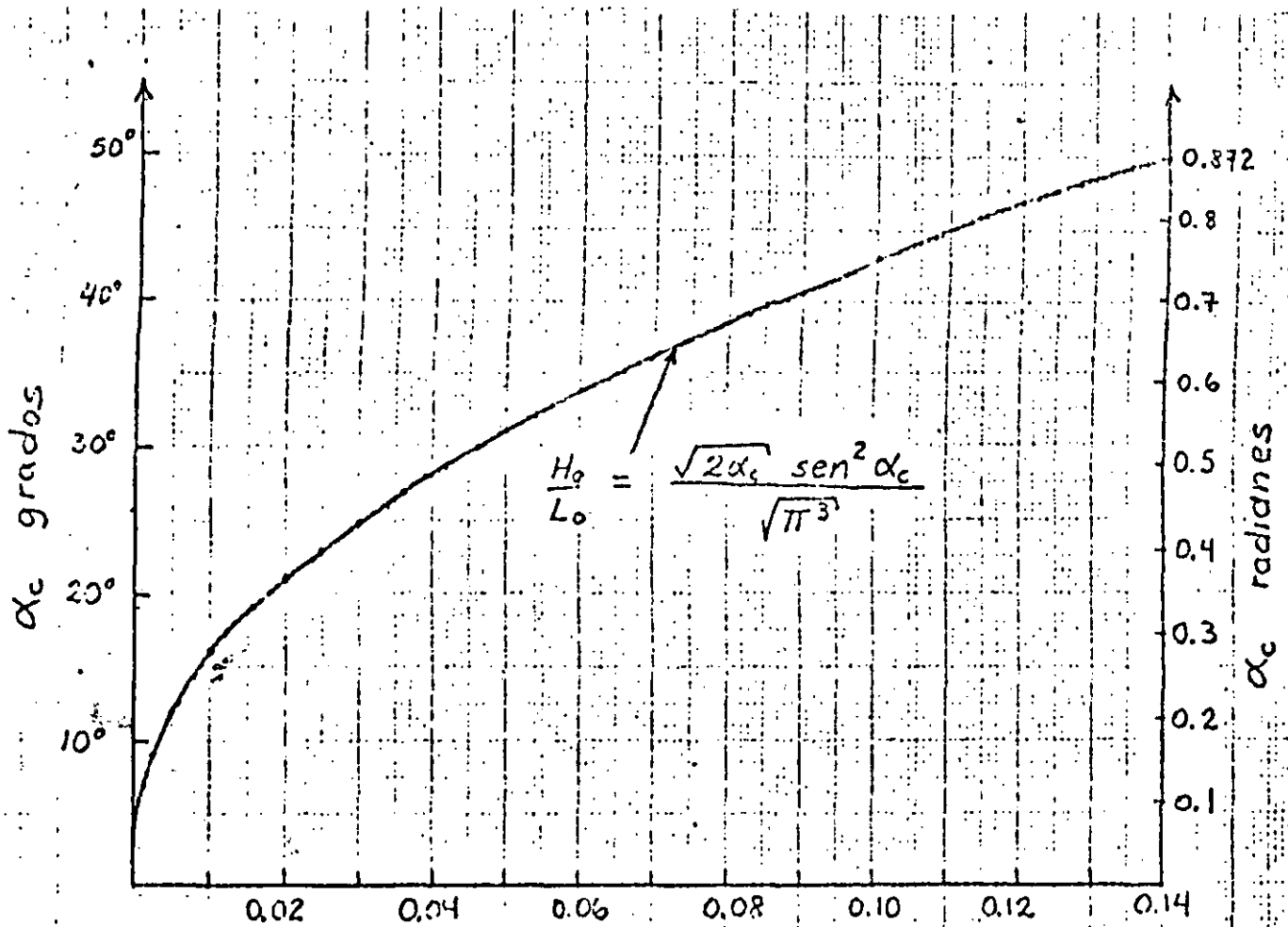
Para los casos ① y ② se utiliza la siguiente expresión

$$R_L = H_0 D_d F \left[ \sqrt{\frac{\pi}{2\alpha}} + \frac{\pi H_0 \coth h}{L} \right]$$

$\alpha$  está en radianes

para utilizarla es necesario utilizar una gráfica para estimar  $\alpha$





Fig

Para obtener el ángulo crítico  $\alpha_c$

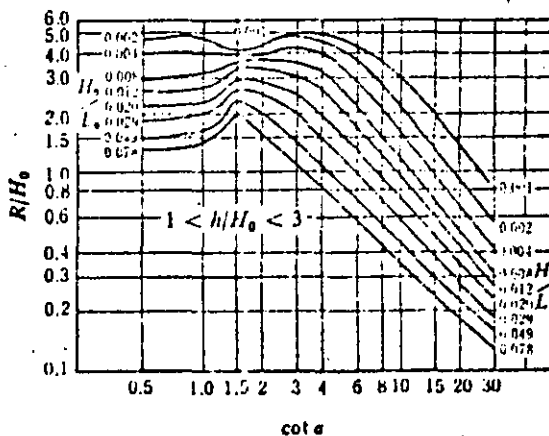


Fig. 1.12.5 Wave run-up diagram used for the hypothetical single slope method (after Saville, 1958).

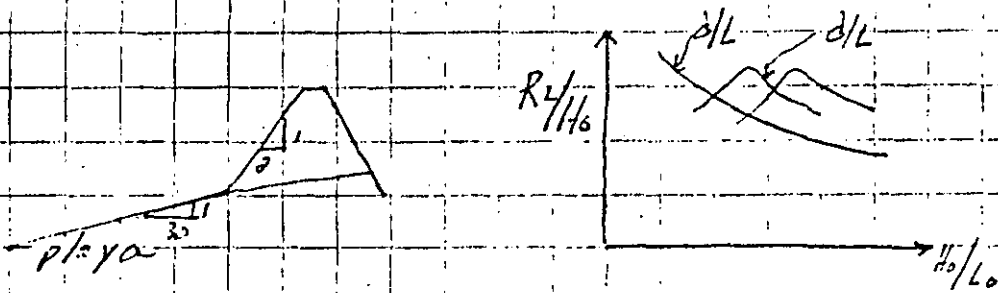
si  $\alpha < \alpha_c$  la ola rompe sobre la estructura.

$$y \quad F = \left( \frac{\cot \alpha_c}{\cot \alpha} \right)^{2/3}$$

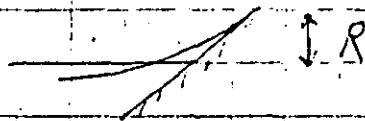
si  $\alpha \geq \alpha_c$  la ola no rompe sobre la estructura.

$$y \quad F = 1$$

Para el caso en que la ola rompe antes de llegar a la estructura el alcance de la ola se define a partir de las Fig. 2.10.3.



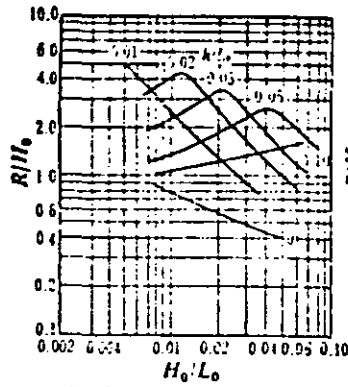
$$R = R_L \beta$$



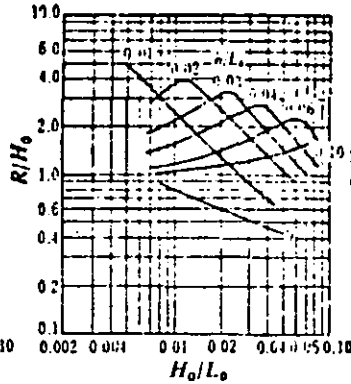
Es necesario revisar si la ola rompe antes de llegar a la estructura.

• Complemente antes de llegar a la estructura

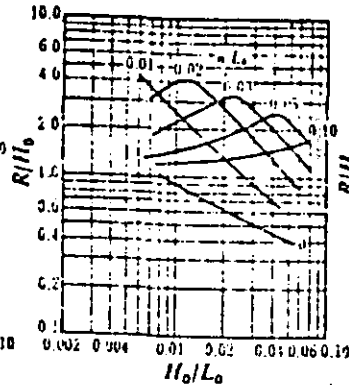
FIG. 2.10.3



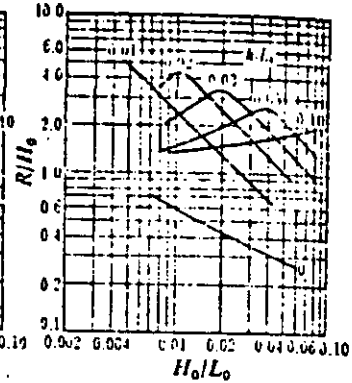
(a) Structure slope 1:0.5,  
Beach slope 1:20



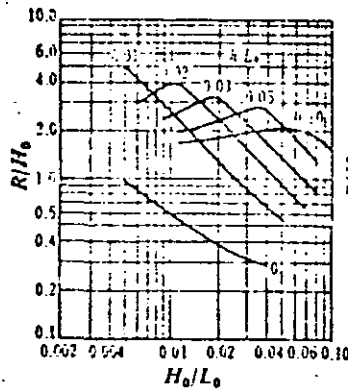
(b) Structure slope 1:0.5,  
Beach slope 1:30



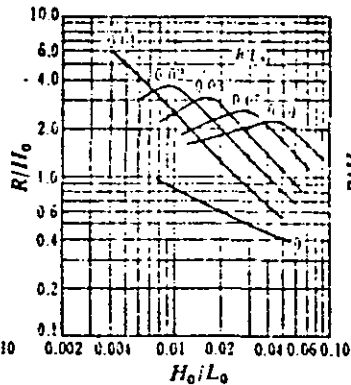
(c) Structure slope 1:1,  
Beach slope 1:20



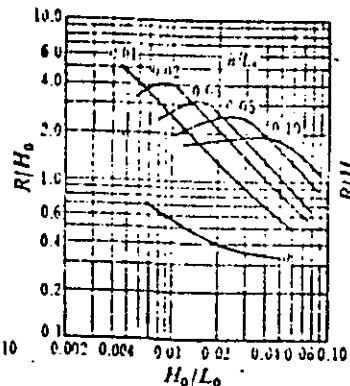
(d) Structure slope 1:1,  
Beach slope 1:30



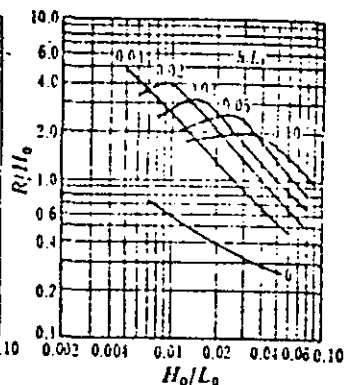
(e) Structure slope 1:2,  
Beach slope 1:20



(f) Structure slope 1:2,  
Beach slope 1:30



(g) Structure slope 1:3,  
Beach slope 1:20



(h) Structure slope 1:3,  
Beach slope 1:30



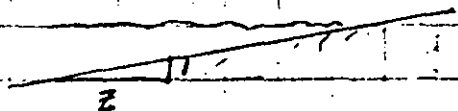
Material y acabado de la superficie	f.
Losa de concreto	1-0.9
Pastos	0.85-0.9
Una capa de rezoga sobre superf. impermeables	0.80
Piedras acomodadas	0.75-0.80
Piedras redondas	0.60-0.65
" colocados a volteo	0.50-0.60
Dos capas de rezoga	0.50
Elementos artificiales	0.50
Mampostería (dos capas)	0.50-0.55
una capa	0.80

★ Alcance de la ola sobre la playa.  
(Run-up o UPRUSH)

$$R = d A (H/d)^B$$

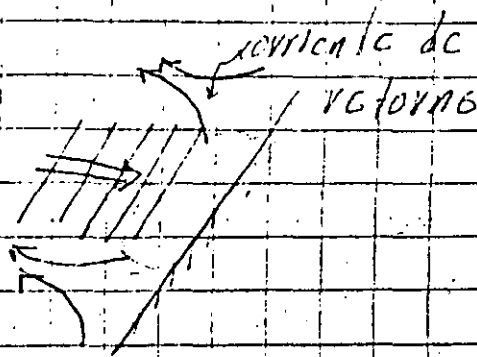
$\alpha$	5°	10°	15°	25°	45°
A	2.15	3.43	3.75	3.35	3.1
B	0.81	1.04	1.12	1.12	1.15

$$\alpha = \text{ang } \tan(1/2)$$

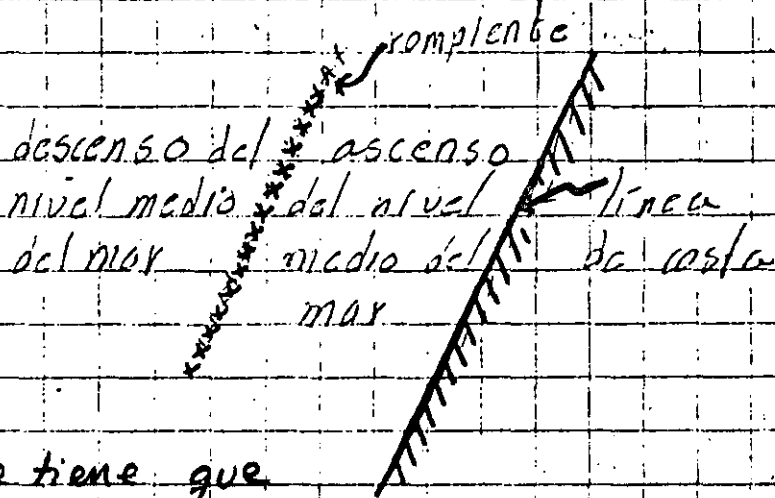


## 2.6 Ascenso y descenso.

Recordando el concepto de Transporte de masa

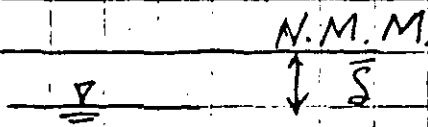


esto se debe a que las partículas si bien a un desplazamiento en aguas poco profundas.

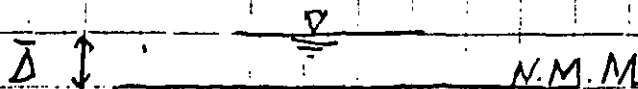


se tiene que

descenso (set-down)  $\bar{\Delta} = \frac{H^2}{8} \frac{k}{\sinh 2kd}$



ascenso (set-up)  $\bar{\Delta} = K(d_b - d) + \Delta_b$



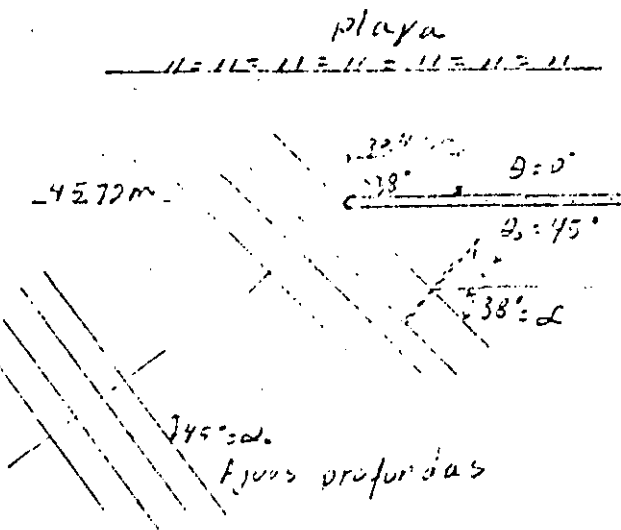
$k = \frac{1}{1 + (8/3.8^2)}$

$\gamma = 0.73$

# Hidráulica Marítima y de Estuarios

## EJEMPLOS

- 1.- Un oleaje tal que en aguas profundas tiene una altura de 10.85 m y un periodo de 12.6 seg. se aproxima a la línea de playa en una dirección de  $45^\circ$ .
- a) En un punto donde la profundidad es 45.72 m las olas han sido afectadas, ¿cual es la altura de ola en ese punto?
- b) Si en ese punto ellas encuentran un rompeolas que es paralelo a la línea de playa ¿cual será la altura del oleaje en el lado protegido a 30.48 m desde el extremo del rompeolas sobre este.



Datos:  $H_0 = 10.85 \text{ m}$ ,  $T = 12.6 \text{ seg}$

$$L_0 = \frac{gT^2}{2\pi} = \frac{9.81(12.6)^2}{2\pi} = 247.87 \text{ m}$$

$$d_e = 0.5 ; d_0 = 0.5(247.87) = 123.94 \text{ m}$$

para  $d = 45.72 \text{ m}$

$$\frac{d}{L_0} = \frac{45.72}{247.87} = 0.1844$$

$$\frac{d}{L} = 0.210 \quad L = 215.66 \text{ m}$$

de tablas  $D_d = 0.9151$  (coef. de fondo)

$$C_0 = \frac{L_0}{T_0} = \frac{247.87}{12.6} = 19.67 \text{ m/seg} ;$$

$$C = \frac{L}{T} = \frac{215.66}{12.6} = 17.16 \text{ m/seg}$$

$$\alpha = \arcsin\left(\frac{C}{C_0} \sin \alpha_0\right)$$

$$\alpha = \arcsin\left(\frac{17.16}{19.67} \sin 45\right)$$

$$\alpha = 38.088^\circ$$

$$\alpha_0 = 45^\circ$$

$$K_0 = \frac{\sqrt{\cos 2\alpha}}{\sqrt{\cos \alpha}} = \frac{\sqrt{\cos 45^\circ}}{\sqrt{\cos 38.088^\circ}} = 0.9478 \quad (\text{coef. de refracción})$$

$$\therefore H = H_0 K_0 D_0 = (10.85)(0.9151)(0.9478)$$

$$\boxed{H = 9.41 \text{ m}} \quad \checkmark$$

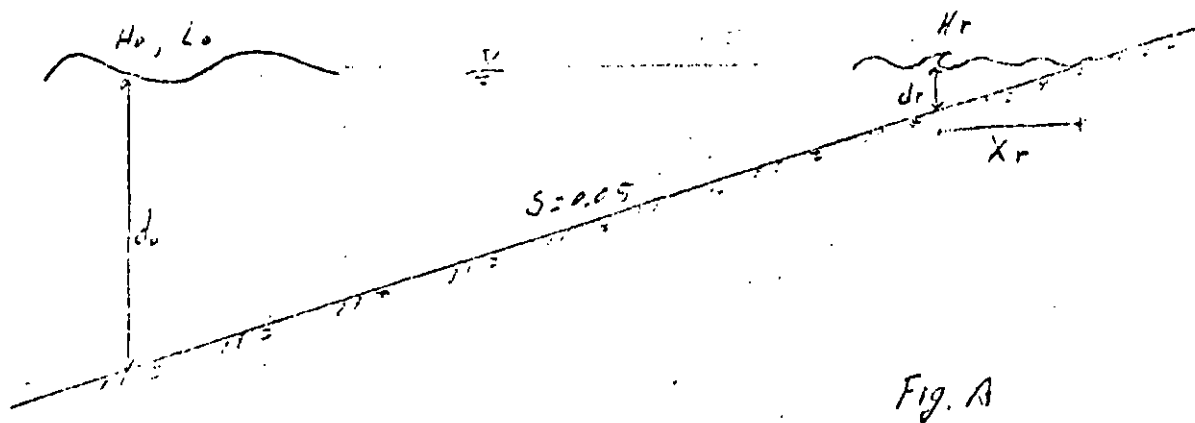
b) Datos  $\theta_0 = 38^\circ$  ;  $\theta = 0^\circ$  ;  $\frac{r}{L} = \frac{30.48}{215.66} = 0.1413$

de la fig  $K_0 = 0.65$  (coef. de difracción)

$$H = K_0 H_i = (0.65)(9.41)$$

$$\boxed{H = 6.12 \text{ m}} \quad \times$$

2. En aguas profundas el oleaje tiene una altura de 5 m y una longitud de 120 m se acerca perpendicularmente a la línea de costa. El fondo marino tiene una pendiente de 0.05. Encontrar la altura rompiente y la profundidad de la rompiente, y la distancia a partir de la línea de costa donde se presenta la rompiente.



Datos:  $H_0 = 5 \text{ m}$ ,  $L_0 = 120 \text{ m}$ ,  $S = 0.05$ ,  $\alpha_0 = 0^\circ$

se propone  $d_r = 6.27 \text{ m}$  (de la fig A  $\alpha = 0^\circ = \alpha_0$ )

$$H_0' = H_0 K_d \quad (K_d = 1) \text{ puesto que } \alpha = \alpha_0; b = b_0$$

$$H_0' = 5(1) = 5 \text{ m}; \quad \frac{H_0'}{L_0} = \frac{5}{120} = 0.0417$$

con  $\frac{H_0'}{L_0} = 0.0417$  y  $S = 0.05$  de la fig. II.3 se obtiene

$$\frac{H_r}{H_0} = 1.17; \quad H_r = 1.17 H_0' = 1.17(5) = 5.85 \text{ m}$$

Revisión

$$T = \sqrt{2\pi L_0/g} = \sqrt{2\pi(120)/9.81} = 8.77 \text{ seg}$$

$$\text{como } \frac{H_r}{gT^2} = \frac{5.85}{9.81(8.77)^2} = 0.0078$$

con  $\frac{H_r}{gT^2} = 0.0078$  y  $S = 0.05$ , de la figura se obtiene

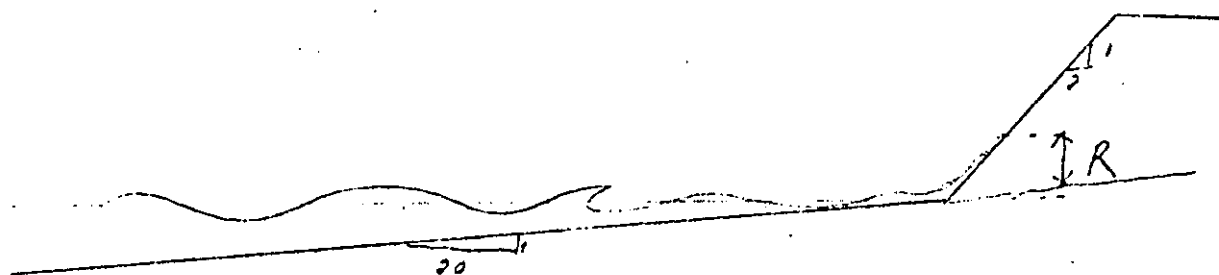
$$\frac{d_r}{H_r} = 1.07; \quad d_r = 1.07 H_r = 1.07(5.85) = 6.26 \text{ m}$$

$$\therefore \begin{array}{l} H_r = 5.85 \text{ m} \quad \checkmark \\ d_r = 6.26 \text{ m} \quad \checkmark \end{array}$$

$$\text{con } \phi = 0.05 = \frac{d_r}{X_r}; \quad X_r = \frac{d_r}{0.05} = \frac{6.26}{0.05}$$

$$\therefore X_r = 125.20 \text{ m} \quad \checkmark$$

3. Determinar el alcance de la ola sobre un muro de piedra redonda que tiene un talud de 2:1; si el oleaje a una profundidad de 40m tiene una altura de 5m y un periodo de 10 seg.



$$L_0 = \frac{gT^2}{2\pi} = \frac{9.81(10)^2}{2\pi} = 156.13 \text{ m} ; \frac{d_0}{L_0} = 0.5 \Rightarrow d_0 = 78.06 \text{ m}$$

$\therefore H = 5 \text{ m}$  pertenece a la condición de aguas intermedias

$$\frac{d}{L_0} = \frac{40}{156.13} = 0.2562 \Rightarrow \frac{d}{L} = 0.2733 \text{ y } L = 146.36 \text{ m}$$

y también  $D_d = 0.9344$  (coeficiente de fonda)

se supone que  $\alpha = \alpha_0$  y  $K_d = 1$  (coef. refracción)

$$H = H_0 D_d K_d ; H_0 = \frac{H}{D_d K_d} = \frac{5}{0.9344} = 5.35 \text{ m}$$

$\therefore$  las condiciones de aguas profundas son  $\begin{cases} H_0 = 5.35 \text{ m} \\ T_0 = 10 \text{ seg} \end{cases}$

A continuación se calculan las condiciones de oleaje rompiente

$$H_o' = H_o K_d = (5.35)(1) = 5.35 \text{ m}$$

$$\frac{H_o'}{L_o} = \frac{5.35}{156.13} = 0.034 \quad \text{y como } s = 0.05$$

de la fig. II.3 se estima  $\frac{H_r}{H_o'} = 1.23$

$$H_r = 1.23 H_o' = 1.23(5.35) = 6.58 \text{ m}$$

Por otro lado

$$\frac{H_r}{gT^2} = \frac{6.58}{9.81(10)^2} = 0.0067 \quad \text{de la fig. II.4 y con } s = 0.05$$

$$\text{se estima } \frac{d_r}{H_r} = 1.04 \quad ; \quad d_r = 1.04(6.58) = 6.88 \text{ m}$$

∴ Las condiciones de oleaje rompiente son

$$H_r = 6.58 \text{ m}$$

$$d_r = 6.88 \text{ m}$$

y la ola rompe antes de llegar a la estructura.

de la fig. 2.10.3 con  $s = 0.05$  y  $Z = 2$  se estima

$$\text{para } \frac{h}{L_o} = 0 \quad \text{y} \quad \frac{H_o}{L_o} = \frac{5.35}{156.13} = 0.0343$$

$$\frac{R_L}{H_o} = 0.3 \quad ; \quad R_L = 0.3(5.35) = 1.6 \text{ m} \quad \text{y } f = 0.65$$

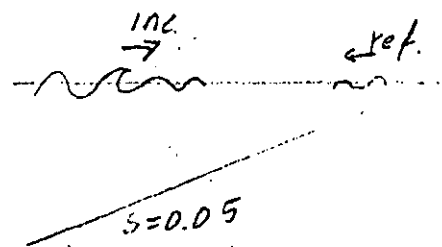
$$\text{como } R = f R_L = (0.65)(1.6)$$

$$R = 1 \text{ m}$$

4. ¿Cual será la altura del oleaje reflejada desde una playa lisa e impermeable con talud 20:1, si el oleaje en aguas profundas tiene una altura de 6 m y un período de 12 seg

Datos:  $H_0 = 6 \text{ m}$        $T = 12 \text{ seg}$

$$L_0 = \frac{gT^2}{2\pi} = \frac{9.81(12)^2}{2\pi} = 224.83 \text{ m}$$



se supone  $K_d = 1$

$$H_0' = H_0 K_d = 6 \text{ m} ; \frac{H_0'}{L_0} = \frac{6}{224.83} = 0.0267$$

de la fig. II.3       $\frac{H_r}{H_0'} = 1.29$        $H_r = 1.29(6) = 7.74 \text{ m}$

con  $\frac{H_r}{gT^2} = \frac{7.74}{9.81(12)^2} = 0.0055$  de la fig. II.4

se estima  $\frac{d_r}{H_r} = 1.032$  ;  $d_r = 1.032(7.74) \approx 8 \text{ m}$

∴ las condiciones de oleaje rompiente son

$$H_r = 7.74 \text{ m}$$

$$d_r = 8 \text{ m}$$

El coeficiente de reflexión es

$$K_r = \alpha_1 \cdot \alpha_2$$

$$\alpha_1 = 0.8 \text{ (playa lisa e imp.)}$$



$$\left(\frac{H_o}{L_o}\right)_{\text{máx}} = \left(\frac{2\theta}{\pi}\right)^{1/2} \frac{\text{sen}^2 \theta}{\pi}$$

$$\theta = \text{ang. } \tan^{-1}(1/20) = 2.8624^\circ = 0.05 \text{ rad}$$

$$\left(\frac{H_o}{L_o}\right)_{\text{máx}} = \left[\frac{2(0.05)}{\pi}\right]^{1/2} \frac{\text{sen}^2 2.8624}{\pi} = 0.0001$$

$$X_2 = \frac{(H_o/L_o)_{\text{máx}}}{(H_o/L_o)} = \frac{0.0001}{(6/224.83)} = 0.0053$$

$$K_L = X_1 \cdot X_2 = (0.8)(0.0053) = 0.0042 \quad \checkmark$$

$$A_{\text{ref}} = K_L A_{\text{inc}} \quad ; \quad A_{\text{inc}} = \frac{1}{2} H_r = \frac{1}{2}(7.74) = 3.87$$

$$A_{\text{ref}} = (0.0042)(3.87) = 0.016 \text{ m}$$

$$\therefore H_{\text{ref}} = 2A_{\text{ref}}$$

$$H_{\text{ref}} = 0.033 \text{ m}$$

5. Defina refracción, difracción y reflexión.

Refracción: Se define como el proceso mediante el cual, las ondas cambian su dirección de propagación a causa de una disminución de la profundidad.

Difracción: Se llama así al proceso en el que la energía de una onda, decae lateralmente debido a la falta de soporte generado al interceptar la esquina de algún obstáculo.

Reflexión: Ocurre por el choque de una onda con algún obstáculo, lo cual da origen a otra nueva onda que parte del obstáculo.

APPENDIX I. TABLE OF FUNCTIONS OF  $d/L_0$

$d/L_0$	$d/L$	$2\pi d/L$	$\sinh 2\pi d/L$	$\sinh 2\pi d/L$	$\cosh 2\pi d/L$	$K$	$4\pi d/L$	$\sinh 4\pi d/L$	$\cosh 4\pi d/L$	$n$	$C_0/C_0$	$H/H_0$	$M$
0	0	0	0	0	1	1	0	0	1	1	0		
.0001000	.003990	.02507	.02506	.02507	1.0003	.9997	.05014	.05016	1.001	.9998	.02506	4.467	7.855
.0002000	.007980	.05014	.05014	.05014	1.0006	.9994	.07091	.07097	1.003	.9996	.03543	3.757	3.928
.0003000	.01197	.07521	.07521	.07521	1.0009	.9991	.08686	.08697	1.004	.9994	.04338	3.395	2.620
.0004000	.01596	.10028	.10028	.10028	1.0013	.9987	.1003	.1005	1.005	.9992	.05007	3.160	1.965
.0005000	.01995	.12542	.12542	.12542	1.0016	.9984	.1122	.1124	1.006	.9990	.05596	2.989	1.572
.0006000	.02394	.15056	.15056	.15056	1.0019	.9981	.1229	.1232	1.008	.9988	.06128	2.556	1.311
.0007000	.02793	.17570	.17570	.17570	1.0022	.9978	.1327	.1331	1.009	.9985	.06617	2.749	1.124
.0008000	.03192	.20084	.20084	.20084	1.0025	.9975	.1419	.1424	1.010	.9983	.07072	2.659	983.5
.0009000	.03591	.22598	.22598	.22598	1.0028	.9972	.1505	.1511	1.011	.9981	.07499	2.582	874.3
.001000	.01263	.07935	.07918	.07943	1.0032	.9969	.1587	.1594	1.013	.9979	.07902	2.515	787.0
.001100	.01325	.08323	.08304	.08333	1.0035	.9966	.1665	.1672	1.014	.9977	.08285	2.456	715.6
.001200	.01384	.08694	.08672	.08705	1.0038	.9962	.1739	.1748	1.015	.9975	.08651	2.404	656.1
.001300	.01440	.09050	.09026	.09063	1.0041	.9959	.1810	.1820	1.016	.9973	.09001	2.357	605.8
.001400	.01495	.09393	.09365	.09407	1.0044	.9956	.1879	.1890	1.018	.9971	.09338	2.314	562.6
.001500	.01548	.09723	.09693	.09739	1.0047	.9953	.1945	.1957	1.019	.9969	.09663	2.275	525
.001600	.01598	.1004	.1001	.1006	1.0051	.9949	.2009	.2022	1.020	.9967	.09977	2.239	493
.001700	.01648	.1035	.1032	.1037	1.0054	.9946	.2071	.2086	1.022	.9965	.1028	2.205	463
.001800	.01696	.1066	.1062	.1068	1.0057	.9943	.2131	.2147	1.023	.9962	.1058	2.174	438
.001900	.01743	.1095	.1091	.1097	1.0060	.9940	.2190	.2207	1.024	.9960	.1087	2.145	415
.002000	.01788	.1123	.1119	.1125	1.0063	.9937	.2247	.2266	1.025	.9958	.1114	2.119	394
.002100	.01832	.1151	.1146	.1154	1.0066	.9934	.2303	.2323	1.027	.9956	.1141	2.094	376
.002200	.01876	.1178	.1173	.1181	1.0069	.9931	.2357	.2379	1.028	.9954	.1161	2.070	359
.002300	.01918	.1205	.1199	.1208	1.0073	.9928	.2410	.2433	1.029	.9952	.1193	2.047	343
.002400	.01959	.1231	.1225	.1234	1.0076	.9925	.2462	.2487	1.031	.9950	.1219	2.025	329
.002500	.02000	.1257	.1250	.1260	1.0079	.9922	.2513	.2540	1.032	.9948	.1243	2.005	316
.002600	.02040	.1282	.1275	.1285	1.0082	.9919	.2563	.2592	1.033	.9946	.1268	1.986	304
.002700	.02079	.1306	.1299	.1310	1.0085	.9916	.2612	.2642	1.034	.9944	.1292	1.967	292
.002800	.02117	.1330	.1323	.1334	1.0089	.9912	.2661	.2692	1.036	.9942	.1315	1.950	282
.002900	.02155	.1354	.1346	.1358	1.0092	.9909	.2708	.2741	1.037	.9939	.1338	1.933	272
.003000	.02192	.1377	.1369	.1382	1.0095	.9906	.2755	.2790	1.038	.9937	.1360	1.917	263
.003100	.02228	.1400	.1391	.1405	1.0098	.9903	.2800	.2837	1.040	.9935	.1382	1.902	255
.003200	.02264	.1423	.1413	.1427	1.0101	.9900	.2845	.2884	1.041	.9933	.1404	1.887	247
.003300	.02300	.1445	.1435	.1449	1.0104	.9897	.2890	.2930	1.042	.9931	.1425	1.873	240
.003400	.02335	.1467	.1456	.1472	1.0108	.9893	.2934	.2976	1.043	.9929	.1446	1.860	233
.003500	.02369	.1488	.1477	.1494	1.0111	.9890	.2977	.3021	1.045	.9927	.1466	1.847	226
.003600	.02403	.1510	.1498	.1515	1.0114	.9887	.3020	.3065	1.046	.9925	.1487	1.834	220
.003700	.02436	.1531	.1519	.1537	1.0117	.9884	.3061	.3109	1.047	.9923	.1507	1.822	214
.003800	.02469	.1551	.1539	.1558	1.0121	.9881	.3103	.3153	1.049	.9921	.1527	1.810	208
.003900	.02502	.1572	.1559	.1579	1.0124	.9878	.3144	.3196	1.050	.9919	.1546	1.799	203
.004000	.02534	.1592	.1579	.1599	1.0127	.9875	.3184	.3238	1.051	.9917	.1565	1.788	198
.004100	.02566	.1612	.1598	.1619	1.0130	.9872	.3224	.3280	1.052	.9915	.1584	1.777	193
.004200	.02597	.1632	.1617	.1639	1.0133	.9869	.3263	.3322	1.054	.9912	.1602	1.767	189
.004300	.02628	.1651	.1636	.1659	1.0137	.9865	.3302	.3362	1.055	.9910	.1621	1.756	184
.004400	.02659	.1671	.1655	.1678	1.0140	.9862	.3341	.3403	1.056	.9908	.1640	1.746	180
.004500	.02689	.1690	.1674	.1698	1.0143	.9859	.3380	.3444	1.058	.9906	.1658	1.737	176
.004600	.02719	.1708	.1692	.1717	1.0146	.9856	.3417	.3483	1.059	.9904	.1676	1.727	172
.004700	.02749	.1727	.1710	.1736	1.0149	.9853	.3454	.3523	1.060	.9902	.1693	1.718	169
.004800	.02778	.1745	.1728	.1754	1.0153	.9849	.3491	.3562	1.062	.9900	.1711	1.709	165
.004900	.02807	.1764	.1746	.1773	1.0156	.9846	.3527	.3601	1.063	.9898	.1728	1.701	162
.005000	.02836	.1782	.1764	.1791	1.0159	.9843	.3564	.3640	1.064	.9896	.1746	1.692	159
.005100	.02864	.1800	.1781	.1809	1.0162	.9840	.3599	.3678	1.066	.9894	.1762	1.684	156
.005200	.02893	.1818	.1798	.1827	1.0166	.9837	.3635	.3715	1.067	.9892	.1779	1.676	153
.005300	.02921	.1835	.1815	.1845	1.0169	.9834	.3670	.3753	1.068	.9889	.1795	1.669	150
.005400	.02948	.1852	.1832	.1863	1.0172	.9831	.3705	.3790	1.069	.9887	.1811	1.662	147
.005500	.02976	.1870	.1848	.1880	1.0175	.9828	.3739	.3827	1.071	.9885	.1827	1.654	145
.005600	.03003	.1887	.1865	.1898	1.0178	.9825	.3774	.3864	1.072	.9883	.1843	1.647	142
.005700	.03030	.1904	.1881	.1915	1.0182	.9822	.3808	.3900	1.073	.9881	.1859	1.640	140
.005800	.03057	.1921	.1897	.1932	1.0185	.9818	.3841	.3937	1.075	.9879	.1874	1.633	137
.005900	.03083	.1937	.1913	.1949	1.0188	.9815	.3875	.3972	1.076	.9877	.1890	1.626	135
.006000	.03110	.1954	.1929	.1967	1.0192	.9812	.3908	.4008	1.077	.9875	.1905	1.620	133
.006100	.03136	.1970	.1945	.1983	1.0195	.9809	.3941	.4044	1.079	.9873	.1920	1.614	130
.006200	.03162	.1987	.1961	.2000	1.0198	.9806	.3973	.4079	1.080	.9871	.1935	1.607	128
.006300	.03188	.2003	.1976	.2016	1.0201	.9803	.4006	.4114	1.081	.9869	.1950	1.601	126
.006400	.03213	.2019	.1992	.2033	1.0205	.9799	.4038	.4148	1.083	.9867	.1965	1.595	124
.006500	.03238	.2035	.2007	.2049	1.0208	.9796	.4070	.4183	1.084	.9865	.1980	1.589	123
.006600	.03264	.2051	.2022	.2065	1.0211	.9793	.4101	.4217	1.085	.9863	.1994	1.583	121
.006700	.03289	.2066	.2037	.2081	1.0214	.9790	.4133	.4251	1.087	.9860	.2009	1.578	119
.006800	.03313	.2082	.2052	.2097	1.0217	.9787	.4164	.4285	1.088	.9858	.2023	1.572	117
.006900	.03338	.2097	.2067	.2113	1.0221	.9784	.4195	.4319	1.089	.9856	.2037	1.567	116

APPENDIX I. (Continued)

$d/L_0$	$d/L$	$2\pi d/L$	$\tanh 2\pi d/L$	$\sinh 2\pi d/L$	$\cosh 2\pi d/L$	$K$	$4\pi d/L$	$\sinh 4\pi d/L$	$\cosh 4\pi d/L$	$n$	$C_0/C_0$	$H/H_0$	$M$
.007000	.03362	.2113	.2082	.2128	1.0224	.9781	.4225	.4352	1.091	.9854	.2051	1.561	114
.007100	.03387	.2128	.2096	.2144	1.0227	.9778	.4256	.4386	1.092	.9852	.2065	1.556	112
.007200	.03411	.2143	.2111	.2160	1.0231	.9774	.4286	.4419	1.093	.9850	.2079	1.551	111
.007300	.03435	.2158	.2125	.2175	1.0234	.9771	.4316	.4452	1.095	.9848	.2093	1.546	109
.007400	.03459	.2173	.2139	.2190	1.0237	.9768	.4346	.4484	1.096	.9846	.2106	1.541	108
.007500	.03482	.2188	.2154	.2205	1.0240	.9765	.4376	.4517	1.097	.9844	.2120	1.536	106
.007600	.03506	.2203	.2168	.2221	1.0244	.9762	.4406	.4549	1.099	.9842	.2134	1.531	105
.007700	.03529	.2218	.2182	.2236	1.0247	.9759	.4435	.4582	1.100	.9840	.2147	1.526	104
.007800	.03552	.2232	.2196	.2251	1.0250	.9756	.4464	.4614	1.101	.9838	.2160	1.521	102
.007900	.03576	.2247	.2209	.2265	1.0253	.9753	.4493	.4646	1.103	.9836	.2173	1.517	101
.008000	.03598	.2261	.2223	.2280	1.0257	.9750	.4522	.4678	1.104	.9834	.2186	1.512	100
.008100	.03621	.2275	.2237	.2295	1.0260	.9747	.4551	.4709	1.105	.9832	.2199	1.508	98.6
.008200	.03644	.2290	.2250	.2310	1.0263	.9744	.4579	.4741	1.107	.9830	.2212	1.503	97.5
.008300	.03666	.2304	.2264	.2324	1.0266	.9741	.4607	.4772	1.108	.9827	.2225	1.499	96.3
.008400	.03689	.2318	.2277	.2338	1.0270	.9737	.4636	.4803	1.109	.9825	.2237	1.495	95.2
.008500	.03711	.2332	.2290	.2353	1.0273	.9734	.4664	.4834	1.111	.9823	.2250	1.491	94.1
.008600	.03733	.2346	.2303	.2367	1.0276	.9731	.4691	.4865	1.112	.9821	.2262	1.487	93.0
.008700	.03755	.2360	.2317	.2381	1.0280	.9728	.4719	.4896	1.113	.9819	.2275	1.482	91.9
.008800	.03777	.2373	.2330	.2396	1.0283	.9725	.4747	.4927	1.115	.9817	.2287	1.478	90.9
.008900	.03799	.2387	.2343	.2410	1.0286	.9722	.4774	.4957	1.116	.9815	.2300	1.474	89.9
.009000	.03821	.2401	.2356	.2424	1.0290	.9718	.4801	.4988	1.118	.9813	.2312	1.471	88.9
.009100	.03842	.2414	.2368	.2438	1.0293	.9715	.4828	.5018	1.119	.9811	.2324	1.467	88.0
.009200	.03864	.2428	.2381	.2452	1.0296	.9712	.4855	.5049	1.120	.9809	.2336	1.463	87.1
.009300	.03885	.2441	.2394	.2465	1.0299	.9709	.4882	.5079	1.122	.9807	.2348	1.459	86.1
.009400	.03906	.2455	.2407	.2479	1.0303	.9706	.4909	.5109	1.123	.9805	.2360	1.456	85.2
.009500	.03928	.2468	.2419	.2493	1.0306	.9703	.4936	.5138	1.124	.9803	.2371	1.452	84.3
.009600	.03949	.2481	.2431	.2507	1.0309	.9700	.4962	.5168	1.126	.9801	.2383	1.448	83.5
.009700	.03970	.2494	.2443	.2520	1.0313	.9697	.4988	.5198	1.127	.9799	.2394	1.445	82.7
.009800	.03990	.2507	.2456	.2534	1.0316	.9694	.5014	.5227	1.128	.9797	.2406	1.442	81.8
.009900	.04011	.2520	.2468	.2547	1.0319	.9691	.5040	.5257	1.130	.9794	.2417	1.438	81.0
.01000	.04032	.2533	.2480	.2560	1.0322	.9688	.5066	.5286	1.131	.9792	.2429	1.435	80.2
.01100	.04233	.2660	.2598	.2691	1.0356	.9656	.5319	.5574	1.145	.9772	.2539	1.403	73.1
.01200	.04426	.2781	.2711	.2817	1.0389	.9625	.5562	.5853	1.159	.9751	.2643	1.375	67.1
.01300	.04612	.2898	.2820	.2938	1.0423	.9594	.5795	.6125	1.173	.9731	.2743	1.350	62.1
.01400	.04791	.3010	.2924	.3056	1.0456	.9564	.6020	.6391	1.187	.9710	.2838	1.327	57.8
.01500	.04964	.3119	.3022	.3170	1.0490	.9533	.6238	.6651	1.201	.9690	.2928	1.307	54.0
.01600	.05132	.3225	.3117	.3281	1.0524	.9502	.6450	.6906	1.215	.9670	.3014	1.288	50.8
.01700	.05296	.3328	.3209	.3389	1.0559	.9471	.6655	.7158	1.230	.9649	.3096	1.271	47.9
.01800	.05455	.3428	.3298	.3495	1.0593	.9440	.6856	.7405	1.244	.9629	.3176	1.255	45.3
.01900	.05611	.3525	.3386	.3599	1.0628	.9409	.7051	.7650	1.259	.9609	.3253	1.240	43.0
.02000	.05763	.3621	.3470	.3701	1.0663	.9378	.7242	.7891	1.274	.9588	.3327	1.226	41.0
.02100	.05912	.3714	.3552	.3800	1.0698	.9348	.7429	.8131	1.289	.9568	.3399	1.213	39.1
.02200	.06057	.3806	.3632	.3898	1.0733	.9317	.7612	.8368	1.304	.9548	.3468	1.201	37.4
.02300	.06200	.3896	.3710	.3995	1.0768	.9287	.7791	.8603	1.319	.9528	.3535	1.189	35.9
.02400	.06340	.3984	.3786	.4090	1.0804	.9256	.7967	.8837	1.335	.9508	.3600	1.178	34.4
.02500	.06478	.4070	.3860	.4184	1.0840	.9225	.8140	.9069	1.350	.9488	.3662	1.168	33.1
.02600	.06613	.4155	.3932	.4276	1.0876	.9195	.8310	.9310	1.366	.9468	.3722	1.159	31.9
.02700	.06747	.4239	.4002	.4367	1.0912	.9164	.8478	.9530	1.381	.9448	.3781	1.150	30.8
.02800	.06878	.4322	.4071	.4457	1.0949	.9133	.8643	.9760	1.397	.9428	.3838	1.141	29.8
.02900	.07007	.4403	.4138	.4546	1.0985	.9103	.8805	.9988	1.413	.9408	.3893	1.133	28.8
.03000	.07135	.4483	.4205	.4634	1.1021	.9073	.8966	1.022	1.430	.9388	.3947	1.125	27.9
.03100	.07260	.4562	.4269	.4721	1.1059	.9042	.9124	1.044	1.446	.9369	.4000	1.118	27.1
.03200	.07385	.4640	.4333	.4808	1.1096	.9012	.9280	1.067	1.462	.9349	.4051	1.111	26.3
.03300	.07507	.4717	.4395	.4894	1.1133	.8982	.9434	1.090	1.479	.9329	.4100	1.104	25.6
.03400	.07630	.4794	.4457	.4980	1.1171	.8952	.9588	1.113	1.496	.9309	.4149	1.098	24.8
.03500	.07748	.4868	.4517	.5064	1.1209	.8921	.9737	1.135	1.513	.9289	.4196	1.092	24.19
.03600	.07867	.4943	.4577	.5147	1.1247	.8891	.9886	1.158	1.530	.9270	.4242	1.086	23.56
.03700	.07984	.5017	.4635	.5230	1.1285	.8861	1.0033	1.180	1.547	.9250	.4287	1.080	22.97
.03800	.08100	.5090	.4691	.5312	1.1324	.8831	1.018	1.203	1.564	.9230	.4330	1.075	22.42
.03900	.08215	.5162	.4747	.5394	1.1362	.8801	1.032	1.226	1.582	.9211	.4372	1.069	21.90
.04000	.08329	.5233	.4802	.5475	1.1401	.8771	1.047	1.248	1.600	.9192	.4414	1.064	21.40
.04100	.08442	.5304	.4857	.5556	1.1440	.8741	1.061	1.271	1.617	.9172	.4455	1.059	20.92
.04200	.08553	.5374	.4911	.5637	1.1479	.8711	1.075	1.294	1.636	.9153	.4495	1.055	20.46
.04300	.08664	.5444	.4964	.5717	1.1518	.8681	1.089	1.317	1.654	.9133	.4534	1.050	20.03
.04400	.08774	.5513	.5015	.5796	1.1558	.8652	1.103	1.340	1.672	.9114	.4571	1.046	19.62
.04500	.08883	.5581	.5066	.5876	1.1599	.8621	1.116	1.363	1.691	.9095	.4607	1.042	19.23
.04600	.08991	.5649	.5116	.5954	1.1639	.8592	1.130	1.386	1.709	.9076	.4643	1.038	18.85
.04700	.09098	.5717	.5166	.6033	1.1679	.8562	1.143	1.409	1.728	.9057	.4679	1.034	18.49
.04800	.09205	.5784	.5215	.6111	1.1720	.8532	1.157	1.433	1.747	.9037	.4713	1.030	18.15
.04900	.09311	.5850	.5263	.6189	1.1760	.8503	1.170	1.456	1.766	.9018	.4746	1.026	17.82

APPENDIX I. (Continued)

$d/L_0$	$d/L$	$2\pi d/L$	$\tanh 2\pi d/L$	$\sinh 2\pi d/L$	$\cosh 2\pi d/L$	$K$	$4\pi d/L$	$\sinh 4\pi d/L$	$\cosh 4\pi d/L$	$n$	$C_0/C_0$	$H/H_0$	$M$
.05000	.09416	.5916	.5310	.6267	1.1802	.8473	1.183	1.479	1.786	.8999	.4779	1.023	17.50
.05100	.09520	.5981	.5357	.6344	1.1843	.8444	1.196	1.503	1.805	.8980	.4811	1.019	17.19
.05200	.09623	.6046	.5403	.6421	1.1884	.8415	1.209	1.526	1.825	.8961	.4842	1.016	16.90
.05300	.09726	.6111	.5449	.6499	1.1926	.8385	1.222	1.550	1.845	.8943	.4873	1.013	16.62
.05400	.09829	.6176	.5494	.6575	1.1968	.8356	1.235	1.574	1.865	.8924	.4903	1.010	16.35
.05500	.09930	.6239	.5538	.6652	1.2011	.8326	1.248	1.598	1.885	.8905	.4932	1.007	16.09
.05600	.1003	.6303	.5582	.6729	1.2053	.8297	1.261	1.622	1.906	.8886	.4960	1.004	15.84
.05700	.1013	.6366	.5626	.6805	1.2096	.8267	1.273	1.646	1.926	.8867	.4988	1.001	15.60
.05800	.1023	.6428	.5668	.6880	1.2138	.8239	1.286	1.670	1.947	.8849	.5015	.9985	15.36
.05900	.1033	.6491	.5711	.6956	1.2181	.8209	1.298	1.695	1.968	.8830	.5042	.9958	15.13
.06000	.1043	.6553	.5753	.7033	1.2225	.8180	1.311	1.719	1.989	.8811	.5068	.9932	14.91
.06100	.1053	.6616	.5794	.7110	1.2270	.8150	1.3231	1.744	2.011	.8792	.5094	.9907	14.70
.06200	.1063	.6678	.5834	.7187	1.2315	.8121	1.336	1.770	2.033	.8773	.5119	.9883	14.50
.06300	.1073	.6739	.5874	.7266	1.2355	.8093	1.348	1.795	2.055	.8755	.5143	.9860	14.30
.06400	.1082	.6799	.5914	.7335	1.2402	.8063	1.360	1.819	2.076	.8737	.5167	.9837	14.11
.06500	.1092	.6860	.5954	.7411	1.2447	.8035	1.372	1.845	2.098	.8719	.5191	.9815	13.92
.06600	.1101	.6920	.5993	.7486	1.2492	.8005	1.384	1.870	2.121	.8700	.5214	.9793	13.74
.06700	.1111	.6981	.6031	.7561	1.2537	.7977	1.396	1.896	2.144	.8682	.5236	.9772	13.57
.06800	.1120	.7037	.6069	.7633	1.2580	.7948	1.408	1.921	2.166	.8664	.5258	.9752	13.40
.06900	.1130	.7099	.6106	.7711	1.2628	.7919	1.420	1.948	2.189	.8646	.5279	.9732	13.24
.07000	.1139	.7157	.6144	.7783	1.2672	.7890	1.432	1.974	2.213	.8627	.5300	.9713	13.08
.07100	.1149	.7219	.6181	.7863	1.2721	.7861	1.444	2.000	2.236	.8609	.5321	.9694	12.92
.07200	.1158	.7277	.6217	.7937	1.2767	.7833	1.455	2.026	2.260	.8591	.5341	.9676	12.77
.07300	.1168	.7336	.6252	.8011	1.2813	.7804	1.467	2.053	2.284	.8572	.5360	.9658	12.62
.07400	.1177	.7395	.6289	.8088	1.2861	.7775	1.479	2.080	2.308	.8554	.5380	.9641	12.48
.07500	.1186	.7453	.6324	.8162	1.2908	.7747	1.490	2.107	2.332	.8537	.5399	.9624	12.34
.07600	.1195	.7511	.6359	.8237	1.2956	.7719	1.502	2.135	2.357	.8519	.5417	.9607	12.21
.07700	.1205	.7569	.6392	.8312	1.3004	.7690	1.514	2.162	2.382	.8501	.5435	.9591	12.08
.07800	.1214	.7625	.6427	.8386	1.3051	.7662	1.525	2.189	2.407	.8483	.5452	.9576	11.95
.07900	.1223	.7683	.6460	.8462	1.3100	.7634	1.537	2.217	2.432	.8465	.5469	.9562	11.83
.08000	.1232	.7741	.6493	.8538	1.3149	.7605	1.548	2.245	2.458	.8448	.5485	.9548	11.71
.08100	.1241	.7799	.6526	.8614	1.3198	.7577	1.560	2.274	2.484	.8430	.5501	.9534	11.59
.08200	.1251	.7854	.6558	.8687	1.3246	.7549	1.571	2.303	2.511	.8413	.5517	.9520	11.47
.08300	.1259	.7911	.6590	.8762	1.3295	.7522	1.583	2.331	2.537	.8395	.5533	.9506	11.36
.08400	.1268	.7967	.6622	.8837	1.3345	.7494	1.594	2.360	2.563	.8378	.5548	.9493	11.25
.08500	.1277	.8026	.6655	.8915	1.3397	.7464	1.605	2.389	2.590	.8360	.5563	.9481	11.14
.08600	.1286	.8080	.6688	.8989	1.3446	.7437	1.616	2.418	2.617	.8342	.5577	.9469	11.04
.08700	.1295	.8137	.6716	.9064	1.3497	.7409	1.628	2.448	2.644	.8325	.5591	.9457	10.94
.08800	.1304	.8193	.6747	.9141	1.3548	.7381	1.639	2.478	2.672	.8308	.5605	.9445	10.84
.08900	.1313	.8250	.6778	.9218	1.3600	.7353	1.650	2.508	2.700	.8290	.5619	.9433	10.74
.09000	.1322	.8306	.6808	.9295	1.3653	.7324	1.661	2.538	2.728	.8273	.5632	.9422	10.65
.09100	.1331	.8363	.6838	.9372	1.3706	.7296	1.672	2.568	2.756	.8255	.5645	.9411	10.55
.09200	.1340	.8420	.6868	.9450	1.3759	.7268	1.684	2.599	2.785	.8238	.5658	.9401	10.46
.09300	.1349	.8474	.6897	.9525	1.3810	.7241	1.695	2.630	2.814	.8221	.5670	.9391	10.37
.09400	.1357	.8528	.6925	.9600	1.3862	.7214	1.706	2.662	2.843	.8204	.5682	.9381	10.29
.09500	.1366	.8583	.6953	.9677	1.3917	.7186	1.717	2.693	2.873	.8187	.5693	.9371	10.21
.09600	.1375	.8639	.6982	.9755	1.3970	.7158	1.728	2.726	2.903	.8170	.5704	.9362	10.12
.09700	.1384	.8694	.7011	.9832	1.4023	.7131	1.739	2.757	2.933	.8153	.5716	.9353	10.04
.09800	.1392	.8749	.7039	.9908	1.4077	.7104	1.750	2.790	2.963	.8136	.5727	.9344	9.962
.09900	.1401	.8803	.7066	.9985	1.4131	.7076	1.761	2.822	2.994	.8120	.5737	.9335	9.884
.1000	.1410	.8858	.7093	1.006	1.4187	.7049	1.772	2.855	3.025	.8103	.5747	.9327	9.808
.1010	.1419	.8913	.7120	1.014	1.4242	.7022	1.783	2.888	3.057	.8086	.5757	.9319	9.734
.1020	.1427	.8967	.7147	1.022	1.4297	.6994	1.793	2.922	3.088	.8069	.5766	.9311	9.661
.1030	.1436	.9023	.7173	1.030	1.4354	.6967	1.805	2.956	3.121	.8052	.5776	.9304	9.590
.1040	.1445	.9076	.7200	1.037	1.4410	.6940	1.815	2.990	3.153	.8036	.5785	.9297	9.519
.1050	.1453	.9130	.7226	1.045	1.4465	.6913	1.826	3.024	3.185	.8019	.5794	.9290	9.451
.1060	.1462	.9184	.7252	1.053	1.4523	.6886	1.837	3.059	3.218	.8003	.5803	.9282	9.384
.1070	.1470	.9239	.7277	1.061	1.4580	.6859	1.848	3.094	3.251	.7986	.5812	.9276	9.318
.1080	.1479	.9293	.7303	1.069	1.4638	.6833	1.858	3.128	3.284	.7970	.5820	.9269	9.254
.1090	.1488	.9343	.7327	1.076	1.4692	.6806	1.869	3.164	3.319	.7954	.5828	.9263	9.191
.1100	.1496	.9400	.7352	1.085	1.4752	.6779	1.880	3.201	3.353	.7937	.5836	.9257	9.129
.1110	.1505	.9456	.7377	1.093	1.4814	.6752	1.891	3.237	3.388	.7920	.5843	.9251	9.068
.1120	.1513	.9508	.7402	1.101	1.4871	.6725	1.902	3.274	3.423	.7904	.5850	.9245	9.009
.1130	.1522	.9563	.7426	1.109	1.4932	.6697	1.913	3.312	3.459	.7888	.5857	.9239	8.950
.1140	.1530	.9616	.7450	1.117	1.4990	.6671	1.923	3.348	3.494	.7872	.5864	.9234	8.891
.1150	.1539	.9670	.7474	1.125	1.5051	.6645	1.934	3.385	3.530	.7856	.5871	.9228	8.835
.1160	.1547	.9720	.7497	1.133	1.5108	.6619	1.944	3.423	3.566	.7840	.5878	.9223	8.780
.1170	.1556	.9775	.7520	1.141	1.5171	.6592	1.955	3.462	3.603	.7824	.5884	.9218	8.726
.1180	.1564	.9827	.7543	1.149	1.5230	.6566	1.966	3.501	3.641	.7808	.5890	.9214	8.673
.1190	.1573	.9882	.7566	1.157	1.5293	.6539	1.977	3.540	3.678	.7792	.5896	.9209	8.621

APPENDIX I. (Continued)

$d/L_0$	$d/L$	$2xd/L$	$\tanh 2xd/L$	$\sinh 2xd/L$	$\cosh 2xd/L$	$K$	$4xd/L$	$\sinh 4xd/L$	$\cosh 4xd/L$	$n$	$C_0/C_0$	$H/H_0$	$M$
.1200	.1581	.9936	.7589	1.165	1.5356	.6512	1.987	3.579	3.716	.7776	.5902	.9204	8.569
.1210	.1590	.9989	.7612	1.174	1.5418	.6486	1.998	3.620	3.755	.7760	.5907	.9200	8.518
.1220	.1598	1.004	.7634	1.182	1.5479	.6460	2.008	3.659	3.793	.7745	.5913	.9196	8.468
.1230	.1607	1.010	.7656	1.190	1.5546	.6433	2.019	3.699	3.832	.7729	.5918	.9192	8.419
.1240	.1615	1.015	.7678	1.198	1.5605	.6407	2.030	3.740	3.871	.7713	.5922	.9189	8.371
.1250	.1624	1.020	.7700	1.207	1.5674	.6381	2.041	3.782	3.912	.7698	.5926	.9186	8.324
.1260	.1632	1.025	.7721	1.215	1.5734	.6356	2.051	3.824	3.952	.7682	.5931	.9182	8.278
.1270	.1640	1.030	.7742	1.223	1.5795	.6331	2.061	3.865	3.992	.7667	.5936	.9178	8.233
.1280	.1649	1.036	.7763	1.231	1.5862	.6305	2.072	3.907	4.033	.7652	.5940	.9175	8.189
.1290	.1657	1.041	.7783	1.240	1.5927	.6279	2.082	3.950	4.074	.7637	.5944	.9172	8.146
.1300	.1665	1.046	.7804	1.248	1.5990	.6254	2.093	3.992	4.115	.7621	.5948	.9169	8.103
.1310	.1674	1.052	.7824	1.257	1.6060	.6228	2.104	4.036	4.158	.7606	.5951	.9166	8.061
.1320	.1682	1.057	.7844	1.265	1.6124	.6202	2.114	4.080	4.201	.7591	.5954	.9164	8.020
.1330	.1691	1.062	.7865	1.273	1.6191	.6176	2.125	4.125	4.245	.7575	.5958	.9161	7.978
.1340	.1699	1.068	.7885	1.282	1.6260	.6150	2.135	4.169	4.288	.7560	.5961	.9158	7.937
.1350	.1708	1.073	.7905	1.291	1.633	.6123	2.146	4.217	4.334	.7545	.5964	.9156	7.897
.1360	.1716	1.078	.7925	1.300	1.640	.6098	2.156	4.262	4.378	.7530	.5967	.9154	7.857
.1370	.1724	1.084	.7945	1.308	1.647	.6073	2.167	4.309	4.423	.7515	.5969	.9152	7.819
.1380	.1733	1.089	.7964	1.317	1.654	.6047	2.177	4.355	4.468	.7500	.5972	.9150	7.781
.1390	.1741	1.094	.7983	1.326	1.660	.6022	2.188	4.402	4.514	.7485	.5975	.9148	7.744
.1400	.1749	1.099	.8002	1.334	1.667	.5998	2.198	4.450	4.561	.7471	.5978	.9146	7.707
.1410	.1758	1.105	.8021	1.343	1.675	.5972	2.209	4.498	4.607	.7456	.5980	.9144	7.671
.1420	.1766	1.110	.8039	1.352	1.681	.5947	2.219	4.546	4.654	.7441	.5982	.9142	7.636
.1430	.1774	1.115	.8057	1.360	1.688	.5923	2.230	4.595	4.663	.7426	.5984	.9141	7.602
.1440	.1783	1.120	.8076	1.369	1.696	.5898	2.240	4.644	4.751	.7412	.5986	.9140	7.567
.1450	.1791	1.125	.8094	1.378	1.703	.5873	2.251	4.695	4.800	.7397	.5987	.9139	7.533
.1460	.1800	1.131	.8112	1.388	1.710	.5847	2.261	4.746	4.850	.7382	.5989	.9137	7.499
.1470	.1808	1.136	.8131	1.397	1.718	.5822	2.272	4.798	4.901	.7368	.5990	.9136	7.465
.1480	.1816	1.141	.8149	1.405	1.725	.5798	2.282	4.847	4.951	.7354	.5992	.9135	7.432
.1490	.1825	1.146	.8166	1.415	1.732	.5773	2.293	4.901	5.001	.7339	.5993	.9134	7.400
.1500	.1833	1.152	.8183	1.424	1.740	.5748	2.303	4.954	5.054	.7325	.5994	.9133	7.369
.1510	.1841	1.157	.8200	1.433	1.747	.5723	2.314	5.007	5.106	.7311	.5994	.9133	7.339
.1520	.1850	1.162	.8217	1.442	1.755	.5699	2.324	5.061	5.159	.7296	.5995	.9132	7.309
.1530	.1858	1.167	.8234	1.451	1.762	.5675	2.335	5.115	5.212	.7282	.5996	.9132	7.279
.1540	.1866	1.173	.8250	1.460	1.770	.5651	2.345	5.169	5.265	.7268	.5996	.9132	7.250
.1550	.1875	1.178	.8267	1.469	1.777	.5627	2.356	5.225	5.320	.7254	.5997	.9131	7.221
.1560	.1883	1.183	.8284	1.479	1.785	.5602	2.366	5.283	5.376	.7240	.5998	.9130	7.191
.1570	.1891	1.188	.8301	1.488	1.793	.5577	2.377	5.339	5.432	.7226	.5999	.9129	7.162
.1580	.1900	1.194	.8317	1.498	1.801	.5552	2.387	5.398	5.490	.7212	.5998	.9130	7.134
.1590	.1908	1.199	.8333	1.507	1.809	.5528	2.398	5.454	5.544	.7198	.5998	.9130	7.107
.1600	.1917	1.204	.8349	1.517	1.817	.5504	2.408	5.513	5.603	.7184	.5998	.9130	7.079
.1610	.1925	1.209	.8365	1.527	1.825	.5480	2.419	5.571	5.660	.7171	.5998	.9130	7.052
.1620	.1933	1.215	.8381	1.536	1.833	.5456	2.429	5.630	5.718	.7157	.5998	.9130	7.026
.1630	.1941	1.220	.8396	1.546	1.841	.5432	2.440	5.690	5.777	.7144	.5998	.9130	7.000
.1640	.1950	1.225	.8411	1.555	1.849	.5409	2.450	5.751	5.837	.7130	.5998	.9130	6.975
.1650	.1958	1.230	.8427	1.565	1.857	.5385	2.461	5.813	5.898	.7117	.5997	.9131	6.949
.1660	.1966	1.235	.8442	1.574	1.865	.5362	2.471	5.874	5.959	.7103	.5996	.9132	6.924
.1670	.1975	1.240	.8457	1.584	1.873	.5339	2.482	5.938	6.021	.7090	.5996	.9132	6.900
.1680	.1983	1.246	.8472	1.594	1.882	.5315	2.492	6.003	6.085	.7076	.5995	.9133	6.876
.1690	.1992	1.251	.8486	1.604	1.890	.5291	2.503	6.066	6.148	.7063	.5994	.9133	6.853
.1700	.2000	1.257	.8501	1.614	1.899	.5267	2.513	6.130	6.212	.7050	.5993	.9134	6.830
.1710	.2008	1.262	.8515	1.624	1.907	.5243	2.523	6.197	6.275	.7036	.5992	.9135	6.807
.1720	.2017	1.267	.8529	1.634	1.915	.5220	2.534	6.262	6.342	.7023	.5991	.9136	6.784
.1730	.2025	1.272	.8544	1.644	1.924	.5197	2.544	6.329	6.407	.7010	.5989	.9137	6.761
.1740	.2033	1.277	.8558	1.654	1.933	.5174	2.555	6.395	6.473	.6997	.5988	.9138	6.738
.1750	.2042	1.282	.8572	1.664	1.941	.5151	2.565	6.465	6.541	.6984	.5987	.9139	6.716
.1760	.2050	1.288	.8586	1.675	1.951	.5127	2.576	6.534	6.610	.6971	.5985	.9140	6.694
.1770	.2058	1.293	.8600	1.685	1.959	.5104	2.586	6.603	6.679	.6958	.5984	.9141	6.672
.1780	.2066	1.298	.8614	1.695	1.968	.5081	2.597	6.672	6.747	.6946	.5982	.9142	6.651
.1790	.2075	1.304	.8627	1.706	1.977	.5058	2.607	6.744	6.818	.6933	.5980	.9144	6.631
.1800	.2083	1.309	.8640	1.716	1.986	.5036	2.618	6.818	6.891	.6920	.5979	.9145	6.611
.1810	.2092	1.314	.8653	1.727	1.995	.5013	2.629	6.890	6.963	.6907	.5977	.9146	6.591
.1820	.2100	1.320	.8666	1.737	2.004	.4990	2.639	6.963	7.035	.6895	.5975	.9148	6.571
.1830	.2108	1.325	.8680	1.748	2.013	.4967	2.650	7.038	7.109	.6882	.5974	.9149	6.550
.1840	.2117	1.330	.8693	1.758	2.022	.4945	2.660	7.113	7.183	.6870	.5972	.9150	6.530
.1850	.2125	1.335	.8706	1.769	2.032	.4922	2.671	7.191	7.260	.6857	.5969	.9152	6.511
.1860	.2134	1.341	.8718	1.780	2.041	.4899	2.681	7.267	7.336	.6845	.5967	.9154	6.492
.1870	.2142	1.346	.8731	1.791	2.051	.4876	2.692	7.345	7.412	.6832	.5965	.9155	6.474
.1880	.2150	1.351	.8743	1.801	2.060	.4854	2.702	7.422	7.488	.6820	.5963	.9157	6.456
.1890	.2159	1.356	.8755	1.812	2.070	.4832	2.712	7.500	7.566	.6808	.5961	.9159	6.438







## APPENDIX I. (Continued)

$d/L_0$	$d/L$	$2\pi d/L$	$\tanh 2\pi d/L$	$\sinh 2\pi d/L$	$\cosh 2\pi d/L$	$K$	$4\pi d/L$	$\sinh 4\pi d/L$	$\cosh 4\pi d/L$	$n$	$C_0/C_0$	$H/H_0$	$M$
.3300	.3394	2.133	.9723	4.139	4.277	.2338	4.263	35.38	35.39	.5599	.5444	.9383	5.220
.3310	.3403	2.138	.9726	4.184	4.301	.2325	4.277	35.99	36.00	.5594	.5441	.9586	5.217
.3320	.3413	2.144	.9729	4.209	4.326	.2312	4.288	36.42	36.43	.5589	.5438	.9589	5.214
.3330	.3422	2.150	.9732	4.234	4.350	.2299	4.300	36.84	36.85	.5584	.5434	.9592	5.210
.3340	.3431	2.156	.9735	4.259	4.375	.2286	4.311	37.25	37.27	.5578	.5431	.9595	5.207
.3350	.3440	2.161	.9738	4.284	4.399	.2273	4.323	37.70	37.72	.5573	.5427	.9598	5.204
.3360	.3449	2.167	.9741	4.310	4.424	.2260	4.335	38.14	38.15	.5568	.5424	.9601	5.201
.3370	.3459	2.173	.9744	4.336	4.450	.2247	4.346	38.59	38.60	.5563	.5421	.9604	5.198
.3380	.3468	2.179	.9747	4.361	4.474	.2235	4.358	39.02	39.04	.5558	.5417	.9607	5.194
.3390	.3477	2.185	.9750	4.388	4.500	.2222	4.369	39.48	39.49	.5553	.5414	.9610	5.191
.3400	.3468	2.190	.9753	4.413	4.525	.2210	4.381	39.95	39.96	.5548	.5411	.9613	5.188
.3410	.3495	2.196	.9756	4.439	4.550	.2198	4.392	40.40	40.41	.5544	.5408	.9615	5.185
.3420	.3504	2.202	.9758	4.466	4.576	.2185	4.404	40.87	40.89	.5539	.5405	.9618	5.182
.3430	.3514	2.208	.9761	4.492	4.602	.2173	4.416	41.36	41.37	.5534	.5402	.9621	5.179
.3440	.3523	2.214	.9764	4.521	4.630	.2160	4.427	41.85	41.84	.5529	.5399	.9623	5.176
.3450	.3532	2.220	.9767	4.547	4.656	.2148	4.439	42.33	42.34	.5524	.5396	.9626	5.173
.3460	.3542	2.225	.9769	4.575	4.682	.2136	4.451	42.83	42.84	.5519	.5392	.9629	5.171
.3470	.3551	2.231	.9772	4.602	4.709	.2124	4.462	43.34	43.35	.5515	.5389	.9632	5.168
.3480	.3560	2.237	.9775	4.629	4.736	.2111	4.474	43.85	43.86	.5510	.5386	.9635	5.165
.3490	.3570	2.243	.9777	4.657	4.763	.2099	4.486	44.37	44.40	.5505	.5383	.9638	5.162
.3500	.3579	2.249	.9780	4.685	4.791	.2087	4.498	44.89	44.80	.5501	.5380	.9640	5.159
.3510	.3588	2.255	.9782	4.713	4.818	.2076	4.509	45.42	45.43	.5496	.5377	.9643	5.157
.3520	.3598	2.260	.9785	4.741	4.845	.2064	4.521	45.95	45.96	.5492	.5374	.9646	5.154
.3530	.3607	2.266	.9787	4.770	4.873	.2052	4.533	46.50	46.51	.5487	.5371	.9648	5.152
.3540	.3616	2.272	.9790	4.798	4.901	.2040	4.544	47.03	47.04	.5483	.5368	.9651	5.149
.3550	.3625	2.278	.9792	4.827	4.929	.2029	4.556	47.59	47.60	.5479	.5365	.9654	5.147
.3560	.3635	2.284	.9795	4.856	4.957	.2017	4.568	48.15	48.16	.5474	.5362	.9657	5.144
.3570	.3644	2.290	.9797	4.885	4.987	.2005	4.579	48.72	48.73	.5470	.5359	.9659	5.141
.3580	.3653	2.296	.9799	4.914	5.015	.1994	4.591	49.29	49.30	.5466	.5356	.9662	5.139
.3590	.3663	2.301	.9801	4.944	5.044	.1983	4.603	49.88	49.89	.5461	.5353	.9665	5.137
.3600	.3672	2.307	.9804	4.974	5.072	.1972	4.615	50.47	50.48	.5457	.5350	.9667	5.134
.3610	.3682	2.313	.9806	5.004	5.103	.1960	4.627	51.08	51.09	.5453	.5347	.9670	5.132
.3620	.3691	2.319	.9808	5.034	5.132	.1949	4.638	51.67	51.67	.5449	.5344	.9673	5.130
.3630	.3700	2.325	.9811	5.063	5.161	.1938	4.650	52.27	52.28	.5445	.5342	.9675	5.127
.3640	.3709	2.331	.9813	5.094	5.191	.1926	4.661	52.89	52.90	.5441	.5339	.9677	5.125
.3650	.3719	2.337	.9815	5.124	5.221	.1915	4.673	53.52	53.53	.5437	.5336	.9680	5.123
.3660	.3728	2.342	.9817	5.155	5.251	.1904	4.685	54.15	54.16	.5433	.5333	.9683	5.121
.3670	.3737	2.348	.9819	5.186	5.281	.1894	4.697	54.78	54.79	.5429	.5330	.9686	5.118
.3680	.3747	2.354	.9821	5.217	5.312	.1883	4.708	55.42	55.43	.5425	.5327	.9688	5.116
.3690	.3756	2.360	.9823	5.248	5.343	.1872	4.720	56.09	56.10	.5421	.5325	.9690	5.114
.3700	.3766	2.366	.9825	5.280	5.374	.1861	4.732	56.76	56.77	.5417	.5322	.9693	5.112
.3710	.3775	2.372	.9827	5.312	5.406	.1850	4.744	57.43	57.44	.5413	.5319	.9696	5.110
.3720	.3785	2.378	.9830	5.345	5.438	.1839	4.756	58.13	58.14	.5409	.5317	.9698	5.107
.3730	.3794	2.384	.9832	5.377	5.469	.1828	4.768	58.82	58.83	.5405	.5314	.9700	5.105
.3740	.3804	2.390	.9834	5.410	5.502	.1818	4.780	59.52	59.53	.5402	.5312	.9702	5.103
.3750	.3813	2.396	.9835	5.443	5.534	.1807	4.792	60.24	60.25	.5398	.5309	.9705	5.101
.3760	.3822	2.402	.9837	5.475	5.566	.1797	4.803	60.95	60.95	.5394	.5306	.9707	5.099
.3770	.3832	2.408	.9839	5.508	5.598	.1786	4.815	61.68	61.68	.5390	.5304	.9709	5.097
.3780	.3841	2.413	.9841	5.541	5.631	.1776	4.827	62.41	62.42	.5387	.5301	.9712	5.095
.3790	.3850	2.419	.9843	5.572	5.661	.1766	4.838	63.13	63.14	.5383	.5299	.9714	5.093
.3800	.3860	2.425	.9845	5.609	5.697	.1756	4.851	63.90	63.91	.5380	.5296	.9717	5.091
.3810	.3869	2.431	.9847	5.643	5.731	.1745	4.862	64.66	64.67	.5376	.5294	.9719	5.090
.3820	.3879	2.437	.9848	5.677	5.765	.1735	4.875	65.45	65.46	.5372	.5291	.9721	5.088
.3830	.3888	2.443	.9850	5.712	5.798	.1725	4.885	66.20	66.21	.5369	.5288	.9724	5.086
.3840	.3898	2.449	.9852	5.746	5.833	.1715	4.898	67.00	67.01	.5365	.5286	.9726	5.084
.3850	.3907	2.455	.9854	5.780	5.866	.1705	4.910	67.80	67.81	.5362	.5284	.9728	5.082
.3860	.3917	2.461	.9855	5.814	5.900	.1695	4.922	68.61	68.62	.5359	.5281	.9730	5.081
.3870	.3926	2.467	.9857	5.850	5.935	.1685	4.934	69.45	69.46	.5355	.5279	.9732	5.079
.3880	.3936	2.473	.9859	5.886	5.970	.1675	4.946	70.28	70.29	.5352	.5276	.9735	5.077
.3890	.3945	2.479	.9860	5.921	6.005	.1665	4.958	71.12	71.13	.5349	.5274	.9737	5.076
.3900	.3955	2.485	.9862	5.957	6.040	.1656	4.970	71.97	71.98	.5345	.5271	.9739	5.074
.3910	.3964	2.491	.9864	5.993	6.076	.1646	4.982	72.85	72.86	.5342	.5269	.9741	5.072
.3920	.3974	2.497	.9865	6.029	6.112	.1636	4.993	73.72	73.72	.5339	.5267	.9743	5.071
.3930	.3983	2.503	.9867	6.066	6.148	.1627	5.005	74.59	74.59	.5336	.5265	.9745	5.069
.3940	.3993	2.509	.9869	6.103	6.185	.1617	5.017	75.48	75.48	.5332	.5262	.9748	5.067
.3950	.4002	2.515	.9870	6.140	6.221	.1608	5.029	76.40	76.40	.5329	.5260	.9750	5.066
.3960	.4012	2.521	.9872	6.177	6.258	.1598	5.041	77.32	77.32	.5326	.5258	.9752	5.064
.3970	.4021	2.527	.9873	6.215	6.295	.1589	5.053	78.24	78.24	.5323	.5255	.9754	5.063
.3980	.4031	2.532	.9874	6.252	6.332	.1579	5.065	79.19	79.19	.5320	.5253	.9756	5.062
.3990	.4040	2.538	.9876	6.290	6.369	.1570	5.077	80.13	80.13	.5317	.5251	.9758	5.060

## APPENDIX I. (Continued)

$d/L_0$	$d/L$	$2xd/L$	$\tanh 2xd/L$	$\sinh 2xd/L$	$\cosh 2xd/L$	$K$	$4xd/L$	$\sinh 4xd/L$	$\cosh 4xd/L$	$n$	$C_0/C_0$	$H/H_0$	$M$
.4000	.4030	2.544	.9877	6.329	6.407	.1561	5.089	81.12	81.12	.5314	.5248	.9761	5.058
.4010	.4059	2.550	.9879	6.367	6.445	.1552	5.101	82.08	82.08	.5311	.5246	.9763	5.056
.4020	.4069	2.556	.9880	6.406	6.483	.1542	5.113	83.06	83.06	.5308	.5244	.9765	5.055
.4030	.4078	2.562	.9882	6.444	6.521	.1533	5.125	84.07	84.07	.5305	.5242	.9766	5.053
.4040	.4088	2.568	.9883	6.484	6.561	.1524	5.137	85.11	85.11	.5302	.5240	.9768	5.052
.4050	.4098	2.575	.9885	6.525	6.601	.1515	5.149	86.14	86.14	.5299	.5238	.9772	5.050
.4060	.4107	2.581	.9886	6.564	6.640	.1506	5.161	87.17	87.17	.5296	.5236	.9772	5.049
.4070	.4116	2.586	.9887	6.603	6.679	.1497	5.173	88.20	88.20	.5293	.5234	.9774	5.048
.4080	.4126	2.592	.9889	6.644	6.718	.1488	5.185	89.28	89.28	.5290	.5232	.9776	5.046
.4090	.4136	2.598	.9890	6.684	6.758	.1480	5.197	90.39	90.39	.5287	.5229	.9778	5.045
.4100	.4145	2.604	.9891	6.725	6.799	.1471	5.209	91.44	91.44	.5285	.5227	.9780	5.044
.4110	.4155	2.610	.9892	6.766	6.839	.1462	5.221	92.55	92.55	.5282	.5225	.9782	5.043
.4120	.4164	2.616	.9894	6.806	6.879	.1454	5.233	93.67	93.67	.5279	.5223	.9784	5.041
.4130	.4174	2.623	.9895	6.849	6.921	.1445	5.245	94.83	94.83	.5277	.5221	.9786	5.040
.4140	.4183	2.629	.9896	6.890	6.963	.1436	5.257	95.96	95.96	.5274	.5219	.9788	5.039
.4150	.4193	2.635	.9898	6.932	7.004	.1428	5.269	97.13	97.13	.5271	.5217	.9790	5.037
.4160	.4203	2.641	.9899	6.974	7.046	.1419	5.281	98.30	98.30	.5269	.5215	.9792	5.036
.4170	.4212	2.647	.9900	7.018	7.088	.1411	5.294	99.52	99.52	.5266	.5213	.9794	5.035
.4180	.4222	2.653	.9901	7.060	7.130	.1403	5.305	100.7	100.7	.5263	.5211	.9795	5.034
.4190	.4231	2.659	.9902	7.102	7.173	.1394	5.317	101.9	101.9	.5261	.5209	.9797	5.033
.4200	.4241	2.665	.9904	7.146	7.215	.1386	5.329	103.1	103.1	.5258	.5208	.9798	5.031
.4210	.4251	2.671	.9905	7.190	7.259	.1378	5.341	104.4	104.4	.5256	.5206	.9800	5.030
.4220	.4260	2.677	.9906	7.234	7.303	.1369	5.353	105.7	105.7	.5253	.5204	.9802	5.029
.4230	.4270	2.683	.9907	7.279	7.349	.1361	5.366	107.0	107.0	.5251	.5202	.9804	5.028
.4240	.4280	2.689	.9908	7.325	7.392	.1353	5.378	108.3	108.3	.5248	.5200	.9806	5.027
.4250	.4289	2.695	.9909	7.371	7.438	.1345	5.390	109.7	109.7	.5246	.5198	.9808	5.026
.4260	.4298	2.701	.9910	7.412	7.479	.1337	5.402	110.9	110.9	.5244	.5196	.9810	5.025
.4270	.4308	2.707	.9911	7.457	7.524	.1329	5.414	112.2	112.2	.5241	.5195	.9811	5.024
.4280	.4318	2.713	.9912	7.503	7.570	.1321	5.426	113.6	113.6	.5239	.5193	.9812	5.023
.4290	.4328	2.719	.9913	7.550	7.616	.1313	5.438	115.0	115.0	.5237	.5191	.9814	5.022
.4300	.4337	2.725	.9914	7.595	7.661	.1305	5.450	116.4	116.4	.5234	.5189	.9816	5.021
.4310	.4347	2.731	.9915	7.642	7.707	.1298	5.462	117.8	117.8	.5232	.5187	.9818	5.020
.4320	.4356	2.737	.9916	7.688	7.753	.1290	5.474	119.2	119.2	.5230	.5186	.9819	5.019
.4330	.4366	2.743	.9917	7.735	7.800	.1282	5.486	120.7	120.7	.5227	.5184	.9821	5.018
.4340	.4376	2.749	.9918	7.783	7.847	.1274	5.499	122.2	122.2	.5225	.5182	.9823	5.017
.4350	.4385	2.755	.9919	7.831	7.895	.1267	5.511	123.7	123.7	.5223	.5181	.9824	5.016
.4360	.4395	2.762	.9920	7.880	7.943	.1259	5.523	125.2	125.2	.5221	.5179	.9826	5.015
.4370	.4405	2.768	.9921	7.922	7.991	.1251	5.535	126.7	126.7	.5218	.5177	.9828	5.014
.4380	.4414	2.774	.9922	7.975	8.035	.1244	5.547	128.3	128.3	.5216	.5176	.9829	5.013
.4390	.4424	2.780	.9923	8.026	8.088	.1236	5.560	129.9	129.9	.5214	.5174	.9830	5.012
.4400	.4434	2.786	.9924	8.075	8.136	.1229	5.572	131.4	131.4	.5212	.5172	.9832	5.011
.4410	.4443	2.792	.9925	8.124	8.185	.1222	5.584	133.0	133.0	.5210	.5171	.9833	5.010
.4420	.4453	2.798	.9926	8.175	8.236	.1214	5.596	134.7	134.7	.5208	.5169	.9835	5.009
.4430	.4463	2.804	.9927	8.228	8.285	.1207	5.608	136.3	136.3	.5206	.5168	.9836	5.008
.4440	.4472	2.810	.9928	8.274	8.334	.1200	5.620	137.9	137.9	.5204	.5166	.9838	5.007
.4450	.4482	2.816	.9929	8.326	8.387	.1192	5.632	139.6	139.6	.5202	.5165	.9839	5.006
.4460	.4492	2.822	.9930	8.379	8.438	.1185	5.644	141.4	141.4	.5200	.5163	.9841	5.005
.4470	.4501	2.828	.9930	8.427	8.486	.1178	5.657	143.1	143.1	.5198	.5161	.9843	5.005
.4480	.4511	2.834	.9931	8.481	8.540	.1171	5.669	144.8	144.8	.5196	.5160	.9844	5.004
.4490	.4521	2.840	.9932	8.532	8.590	.1164	5.681	146.6	146.6	.5194	.5158	.9846	5.003
.4500	.4531	2.847	.9933	8.585	8.643	.1157	5.693	148.4	148.4	.5192	.5157	.9847	5.002
.4510	.4540	2.853	.9934	8.638	8.695	.1150	5.705	150.2	150.2	.5190	.5156	.9848	5.001
.4520	.4550	2.859	.9935	8.693	8.750	.1143	5.717	152.1	152.1	.5188	.5154	.9849	5.000
.4530	.4560	2.865	.9935	8.747	8.804	.1136	5.730	154.0	154.0	.5186	.5152	.9851	5.000
.4540	.4569	2.871	.9936	8.797	8.854	.1129	5.742	155.9	155.9	.5184	.5151	.9852	4.999
.4550	.4579	2.877	.9937	8.853	8.910	.1122	5.754	157.7	157.7	.5182	.5150	.9853	4.998
.4560	.4589	2.883	.9938	8.910	8.965	.1115	5.766	159.7	159.7	.5181	.5148	.9855	4.997
.4570	.4599	2.890	.9938	8.965	9.021	.1109	5.779	161.7	161.7	.5179	.5146	.9857	4.997
.4580	.4608	2.896	.9939	9.016	9.072	.1102	5.791	163.6	163.6	.5177	.5145	.9858	4.996
.4590	.4618	2.902	.9940	9.074	9.129	.1095	5.803	165.6	165.6	.5175	.5144	.9859	4.995
.4600	.4628	2.908	.9941	9.132	9.186	.1089	5.815	167.7	167.7	.5173	.5143	.9860	4.994
.4610	.4637	2.914	.9941	9.183	9.238	.1083	5.827	169.7	169.7	.5172	.5141	.9862	4.994
.4620	.4647	2.920	.9942	9.242	9.296	.1076	5.840	171.8	171.8	.5170	.5140	.9863	4.993
.4630	.4657	2.926	.9943	9.301	9.354	.1069	5.852	173.9	173.9	.5168	.5139	.9864	4.992
.4640	.4666	2.932	.9944	9.353	9.406	.1063	5.864	176.0	176.0	.5167	.5138	.9865	4.991
.4650	.4676	2.938	.9944	9.413	9.466	.1056	5.876	178.2	178.2	.5165	.5136	.9867	4.991
.4660	.4686	2.944	.9945	9.472	9.525	.1050	5.888	180.4	180.4	.5163	.5135	.9868	4.990
.4670	.4695	2.951	.9946	9.533	9.585	.1043	5.900	182.6	182.6	.5162	.5134	.9869	4.989
.4680	.4705	2.957	.9946	9.586	9.638	.1037	5.912	184.8	184.8	.5160	.5132	.9871	4.989
.4690	.4715	2.963	.9947	9.647	9.699	.1031	5.925	187.2	187.2	.5158	.5131	.9872	4.988

## APPENDIX I. (Continued)

$d/L$	$d/L$	$2\pi d/L$	$\sinh 2\pi d/L$	$\sinh 2\pi d/L$	$\cosh 2\pi d/L$	$K$	$4\pi d/L$	$\sinh 4\pi d/L$	$\cosh 4\pi d/L$	$n$	$Cc/C_0$	$H/H_0$	$M$
.4700	.4725	2.969	.9947	9.709	9.760	.1025	5.937	189.5	189.5	.5157	.5129	.9873	4.988
.4710	.4735	2.975	.9948	9.770	9.821	.1018	5.949	191.8	191.8	.5155	.5128	.9874	4.987
.4720	.4744	2.981	.9949	9.826	9.877	.1012	5.962	194.2	194.2	.5154	.5127	.9875	4.986
.4730	.4754	2.987	.9949	9.888	9.938	.1006	5.974	196.5	196.5	.5152	.5126	.9876	4.986
.4740	.4764	2.993	.9950	9.951	10.00	.1000	5.986	199.0	199.0	.5150	.5125	.9877	4.985
.4750	.4774	2.999	.9951	10.01	10.07	.09942	5.999	201.4	201.4	.5149	.5124	.9878	4.984
.4760	.4783	3.005	.9951	10.07	10.12	.09882	6.011	203.9	203.9	.5147	.5122	.9880	4.984
.4770	.4793	3.012	.9952	10.13	10.18	.09820	6.023	206.5	206.5	.5146	.5121	.9881	4.983
.4780	.4803	3.018	.9952	10.20	10.25	.09759	6.036	209.0	209.0	.5144	.5120	.9882	4.983
.4790	.4813	3.024	.9953	10.26	10.31	.09698	6.048	211.7	211.7	.5143	.5119	.9883	4.982
.4800	.4822	3.030	.9953	10.32	10.37	.09641	6.060	214.2	214.2	.5142	.5117	.9885	4.982
.4810	.4832	3.036	.9954	10.39	10.43	.09581	6.072	216.8	216.8	.5140	.5116	.9886	4.981
.4820	.4842	3.042	.9955	10.45	10.50	.09523	6.085	219.5	219.5	.5139	.5115	.9887	4.980
.4830	.4852	3.049	.9955	10.52	10.57	.09464	6.097	222.2	222.2	.5137	.5114	.9888	4.980
.4840	.4862	3.055	.9956	10.59	10.63	.09405	6.109	225.0	225.0	.5136	.5113	.9889	4.979
.4850	.4871	3.061	.9956	10.65	10.69	.09352	6.121	228.3	228.3	.5134	.5112	.9890	4.979
.4860	.4881	3.067	.9957	10.71	10.76	.09294	6.134	230.6	230.6	.5133	.5111	.9891	4.978
.4870	.4891	3.073	.9957	10.78	10.83	.09236	6.146	233.5	233.5	.5132	.5110	.9892	4.978
.4880	.4901	3.079	.9958	10.85	10.90	.09178	6.159	236.4	236.4	.5130	.5109	.9893	4.977
.4890	.4911	3.086	.9958	10.92	10.96	.09121	6.171	239.6	239.6	.5129	.5107	.9895	4.977
.4900	.4920	3.092	.9959	10.99	11.03	.09064	6.183	242.3	242.3	.5128	.5106	.9896	4.976
.4910	.4930	3.098	.9959	11.05	11.09	.09010	6.195	245.2	245.2	.5126	.5105	.9897	4.976
.4920	.4940	3.104	.9960	11.12	11.16	.08956	6.208	248.3	248.3	.5125	.5104	.9898	4.975
.4930	.4950	3.110	.9960	11.19	11.24	.08901	6.220	251.3	251.3	.5124	.5103	.9899	4.975
.4940	.4960	3.117	.9961	11.26	11.31	.08845	6.232	254.5	254.5	.5122	.5102	.9899	4.974
.4950	.4969	3.122	.9961	11.32	11.37	.08793	6.245	257.6	257.6	.5121	.5101	.9900	4.974
.4960	.4979	3.128	.9962	11.40	11.44	.08741	6.257	260.8	260.8	.5120	.5100	.9901	4.973
.4970	.4989	3.135	.9962	11.47	11.51	.08691	6.269	264.0	264.0	.5119	.5099	.9902	4.973
.4980	.4999	3.141	.9963	11.54	11.59	.08637	6.282	267.3	267.3	.5118	.5098	.9903	4.972
.4990	.5009	3.147	.9963	11.61	11.65	.08584	6.294	270.6	270.6	.5116	.5097	.9904	4.972
.5000	.5018	3.153	.9964	11.68	11.72	.08530	6.306	274.0	274.0	.5115	.5096	.9905	4.971
.5010	.5028	3.159	.9964	11.75	11.80	.08477	6.319	277.5	277.5	.5114	.5095	.9905	4.971
.5020	.5038	3.166	.9964	11.83	11.87	.08424	6.331	280.8	280.8	.5113	.5094	.9907	4.971
.5030	.5048	3.172	.9965	11.91	11.95	.08371	6.343	284.3	284.3	.5112	.5093	.9908	4.970
.5040	.5058	3.178	.9965	11.98	12.02	.08320	6.356	287.9	287.9	.5110	.5092	.9909	4.970
.5050	.5067	3.184	.9966	12.05	12.09	.08270	6.368	291.4	291.4	.5109	.5092	.9909	4.969
.5060	.5077	3.190	.9966	12.12	12.16	.08220	6.380	295.0	295.0	.5108	.5091	.9910	4.969
.5070	.5087	3.196	.9967	12.20	12.24	.08169	6.393	298.7	298.7	.5107	.5090	.9911	4.968
.5080	.5097	3.203	.9967	12.28	12.32	.08119	6.405	302.4	302.4	.5106	.5089	.9912	4.968
.5090	.5107	3.209	.9968	12.33	12.39	.08068	6.417	306.2	306.2	.5105	.5088	.9913	4.967
.5100	.5117	3.215	.9968	12.43	12.47	.08022	6.430	310.0	310.0	.5104	.5087	.9914	4.967
.5110	.5126	3.221	.9968	12.50	12.54	.07972	6.442	313.8	313.8	.5103	.5086	.9915	4.967
.5120	.5136	3.227	.9969	12.58	12.62	.07922	6.454	317.7	317.7	.5102	.5085	.9915	4.966
.5130	.5146	3.233	.9969	12.66	12.70	.07873	6.467	321.7	321.7	.5101	.5085	.9916	4.966
.5140	.5156	3.240	.9970	12.74	12.78	.07824	6.479	325.7	325.7	.5100	.5084	.9917	4.965
.5150	.5166	3.246	.9970	12.82	12.86	.07776	6.491	329.7	329.7	.5098	.5083	.9918	4.965
.5160	.5176	3.252	.9970	12.90	12.94	.07729	6.504	333.8	333.8	.5097	.5082	.9919	4.965
.5170	.5185	3.258	.9971	12.98	13.02	.07682	6.516	337.9	337.9	.5096	.5082	.9919	4.964
.5180	.5195	3.264	.9971	13.06	13.10	.07634	6.529	342.2	342.2	.5095	.5081	.9920	4.964
.5190	.5205	3.270	.9971	13.14	13.18	.07587	6.541	346.4	346.4	.5094	.5080	.9921	4.964
.5200	.5215	3.277	.9972	13.22	13.26	.07540	6.553	350.7	350.7	.5093	.5079	.9922	4.963
.5210	.5225	3.283	.9972	13.31	13.35	.07494	6.566	355.1	355.1	.5092	.5078	.9923	4.963
.5220	.5235	3.289	.9972	13.39	13.43	.07449	6.578	359.6	359.6	.5092	.5077	.9924	4.963
.5230	.5244	3.295	.9973	13.47	13.51	.07404	6.590	364.0	364.0	.5091	.5077	.9924	4.962
.5240	.5254	3.301	.9973	13.55	13.59	.07358	6.603	368.5	368.5	.5090	.5076	.9925	4.962
.5250	.5264	3.308	.9973	13.64	13.68	.07312	6.615	373.1	373.1	.5089	.5075	.9926	4.962
.5260	.5274	3.314	.9974	13.73	13.76	.07266	6.628	377.8	377.8	.5088	.5074	.9927	4.961
.5270	.5284	3.320	.9974	13.81	13.85	.07221	6.640	382.5	382.5	.5087	.5074	.9927	4.961
.5280	.5294	3.326	.9974	13.90	13.94	.07177	6.652	387.3	387.3	.5086	.5073	.9928	4.961
.5290	.5304	3.333	.9975	13.99	14.02	.07134	6.665	392.2	392.2	.5085	.5072	.9929	4.960
.5300	.5314	3.339	.9975	14.07	14.10	.07091	6.677	397.0	397.0	.5084	.5071	.9930	4.960
.5310	.5323	3.345	.9975	14.16	14.19	.07047	6.690	402.0	402.0	.5083	.5070	.9931	4.960
.5320	.5333	3.351	.9976	14.25	14.28	.07003	6.702	406.9	406.9	.5082	.5070	.9931	4.959
.5330	.5343	3.357	.9976	14.34	14.37	.06959	6.714	412.0	412.0	.5082	.5069	.9932	4.959
.5340	.5353	3.363	.9976	14.43	14.46	.06915	6.727	417.2	417.2	.5081	.5068	.9933	4.959
.5350	.5363	3.370	.9976	14.52	14.55	.06872	6.739	422.4	422.4	.5080	.5068	.9933	4.959
.5360	.5373	3.376	.9977	14.61	14.64	.06829	6.752	427.7	427.7	.5079	.5067	.9934	4.958
.5370	.5383	3.382	.9977	14.70	14.73	.06787	6.764	433.1	433.1	.5078	.5066	.9935	4.958
.5380	.5393	3.388	.9977	14.79	14.82	.06746	6.776	438.5	438.5	.5077	.5066	.9935	4.958
.5390	.5402	3.394	.9977	14.88	14.91	.06705	6.789	444.0	444.0	.5077	.5065	.9936	4.958



## APPENDIX I. (Continued)

$d/L_0$	$d/L$	$2\pi d/L$	$\tanh \frac{2\pi d}{L}$	$\sinh \frac{2\pi d}{L}$	$\cosh \frac{2\pi d}{L}$	$K$	$4\pi d/L$	$\sinh \frac{4\pi d}{L}$	$\cosh \frac{4\pi d}{L}$	$n$	$C_0/C_0$	$H/H_0$	$M$
.7000	.7002	4.400	.9997	40.71	40.72	.02456	8.799	3,314	3,314	.5013	.5012	.9988	4.938
.7100	.7102	4.462	.9997	43.34	43.35	.02307	8.925	3,757	3,757	.5012	.5011	.9989	4.937
.7200	.7202	4.525	.9998	46.14	46.15	.02167	9.050	4,258	4,258	.5011	.5010	.9990	4.937
.7300	.7302	4.588	.9998	49.13	49.14	.02035	9.175	4,828	4,828	.5010	.5009	.9991	4.937
.7400	.7401	4.650	.9998	52.31	52.32	.01911	9.301	5,473	5,473	.5009	.5008	.9992	4.937
.7500	.7501	4.713	.9998	55.70	55.71	.01795	9.426	6,204	6,204	.5008	.5007	.9993	4.936
.7600	.7601	4.776	.9999	59.30	59.31	.01686	9.552	7,034	7,034	.5007	.5006	.9994	4.936
.7700	.7701	4.839	.9999	63.15	63.16	.01583	9.677	7,976	7,976	.5006	.5005	.9995	4.936
.7800	.7801	4.902	.9999	67.24	67.25	.01487	9.803	9,042	9,042	.5005	.5004	.9996	4.936
.7900	.7901	4.964	.9999	71.60	71.60	.01397	9.929	10,250	10,250	.5005	.5004	.9996	4.936
.8000	.8001	5.027	.9999	76.24	76.24	.01312	10.05	11,620	11,620	.5004	.5004	.9996	4.936
.8100	.8101	5.090	.9999	81.19	81.19	.01232	10.18	13,180	13,180	.5004	.5004	.9996	4.936
.8200	.8201	5.153	.9999	86.44	86.44	.01157	10.31	14,940	14,940	.5003	.5003	.9997	4.935
.8300	.8301	5.215	.9999	92.05	92.05	.01086	10.43	17,340	17,340	.5003	.5003	.9997	4.935
.8400	.8400	5.278	1.000	98.01	98.01	.01020	10.56	19,210	19,210	.5003	.5003	.9997	4.935
.8500	.8500	5.341	1.000	104.4	104.4	.009582	10.68	21,780	21,780	.5002	.5002	.9998	4.935
.8600	.8600	5.404	1.000	111.1	111.1	.009000	10.81	24,690	24,690	.5002	.5002	.9998	4.935
.8700	.8700	5.467	1.000	118.3	118.3	.008451	10.93	28,000	28,000	.5002	.5002	.9998	4.935
.8800	.8800	5.529	1.000	126.0	126.0	.007934	11.06	31,750	31,750	.5002	.5002	.9998	4.935
.8900	.8900	5.592	1.000	134.2	134.2	.007454	11.18	36,000	36,000	.5002	.5002	.9998	4.935
.9000	.9000	5.655	1.000	142.9	142.9	.007000	11.31	40,810	40,810	.5001	.5001	.9999	4.935
.9100	.9100	5.718	1.000	152.1	152.1	.006574	11.44	46,280	46,280	.5001	.5001	.9999	4.935
.9200	.9200	5.781	1.000	162.0	162.0	.006173	11.56	52,470	52,470	.5001	.5001	.9999	4.935
.9300	.9300	5.844	1.000	172.5	172.5	.005797	11.69	59,500	59,500	.5001	.5001	.9999	4.935
.9400	.9400	5.906	1.000	183.7	183.7	.005445	11.81	67,470	67,470	.5001	.5001	.9999	4.935
.9500	.9500	5.969	1.000	195.6	195.6	.005113	11.94	76,490	76,490	.5001	.5001	.9999	4.935
.9600	.9600	6.032	1.000	203.5	203.5	.004914	12.06	86,740	86,740	.5001	.5001	.9999	4.935
.9700	.9700	6.095	1.000	222.8	222.8	.004489	12.19	98,350	98,350	.5001	.5001	.9999	4.935
.9800	.9800	6.158	1.000	236.1	236.1	.004235	12.32	111,500	111,500	.5001	.5001	.9999	4.935
.9900	.9900	6.220	1.000	251.4	251.4	.003977	12.44	126,500	126,500	.5000	.5000	1.000	4.935
1.000	1.000	6.283	1.000	267.7	267.7	.003735	12.57	143,400	143,400	.5000	.5000	1.000	4.935



**DIVISION DE EDUCACION CONTINUA  
FACULTAD DE INGENIERIA U.N.A.M.**

CURSO: OBRAS MARITIMAS  
"HIDRAULICA MARITIMA Y DE ESTUARIOS"  
DEL 11 DE MARZO AL 3 DE JUNIO  
MEXICO, D.F.

O N D A S

ING. OSCAR FUENTES MARILES  
JUNIO 1985

HIDRAULICA MARITIMA Y DE ESTUARIOS

OBJETIVO:

Proporcionar al alumno los fundamentos teóricos de los fenómenos marítimos, con objeto de que los pueda aplicar en la solución de problemas de defensa y aprovechamiento, así como a la investigación.

PROGRAMA

1. ONDAS

- 1.1 Ondas de pequeña amplitud
- 1.2 Ondas de amplitud finita
- 1.3 Otras teorías de ondas

2. TRANSFORMACIONES DE LAS ONDAS

- 2.1 Debido a profundidad somera y a amortiguamiento
- 2.2 Refracción
- 2.3 Difracción
- 2.4 Reflexión
- 2.5 Rompiente
- 2.6 Ascenso y descenso

3. ONDAS OCEANICAS
  - 3.1 Estadística de ondas a corto plazo
  - 3.2 Estadística de ondas a largo plazo
  - 3.3 Oleaje de diseño
  
4. PREDICCIÓN DEL OLEAJE
  - 4.1 Generación y desarrollo de las olas de viento
  - 4.2 Elementos meteorológicos y factores descriptivos
  - 4.3 Métodos de predicción
  
5. EFECTO DEL OLEAJE SOBRE ESTRUCTURAS MARITIMAS
  - 5.1 Pilas
  - 5.2 Muros
  - 5.3 Escolleras y espigones
  
6. PROCESOS COSTEROS
  - 6.1 Playa
  - 6.2 Corrientes
  - 6.3 Régimen de costas
  
7. ESTUARIOS
  - 7.1 Hidrología de costas
  - 7.2 Cuña salina
  - 7.3 Procesos de mezclado
  - 7.4 Dispersión y sedimentación



8. ONDAS DE LARGO PERIODO 3
- 8.1 Oscilaciones en canales y bahías
  - 8.2 Mareas
  - 8.3 Otras clases de ondas

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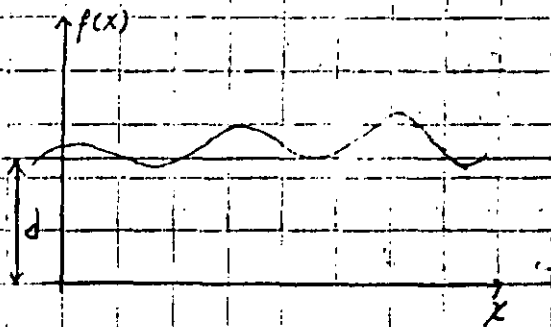
1. ONDAS

La finalidad del curso consiste en:

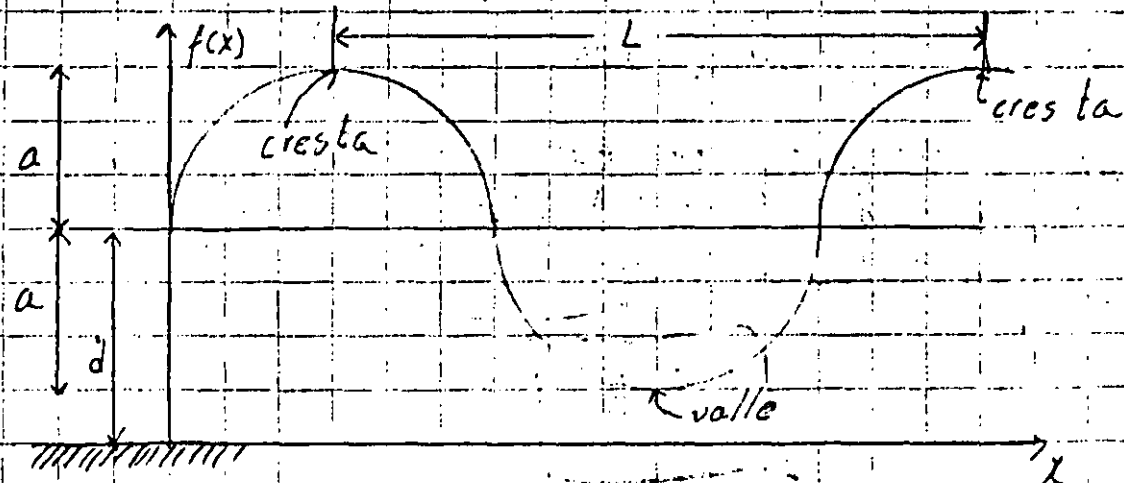
1. Entender la naturaleza de algunos fenómenos costeros, originados por oleaje y corrientes.
2. Desarrollo de técnicas de medición de las características deseadas.
3. Aclarar los mecanismos de sedimentación.
4. Investigación para lograr diseños efectivos y prácticos de construcción adecuados.
5. Entender algunos de los problemas de difusión por mezclado de agua caliente.

Introducción

El oleaje es generado por el viento, atracción del sol y la luna, ó por sismo.



En un principio, para simular el oleaje se utilizó la función trigonométrica del seno.



$a$ , es la amplitud de la onda y es la máxima elongación que puede tener esta.

$H=2a$ , es la altura de la ola

$f$ , es la frecuencia, se mide en (1/seg) ó hz.

El tiempo que transcurre entre el paso de dos crestas consecutivas es el periodo del oleaje

$$T = \frac{1}{f}$$

$1 \leq T \leq 30 \text{ seg.}$   
(ondas de gravedad)

Para simular el oleaje se utiliza la expresión

$$f(t) = a \sin[2\pi f t + \phi]$$

donde  $\phi$  es el ángulo de fase.

al oleaje simulado con esa expresión se le llama senoidal, monocromático o armónico.

Para un instante  $t = t_1$ , si se toma en cuenta la longitud  $L$  del oleaje, se tiene

$$g(x) = a \sin(2\pi \gamma_2 t + \phi)$$

para otro instante  $t = t_2$ , la ola ha cambiado de posición ( $t_2 > t_1$ ), con una celeridad  $c$  que se calcula como

$$c = \frac{L}{T}$$

Se define como frecuencia angular  $\omega$

$$\omega = 2\pi f = \frac{2\pi}{T}$$

se define como número de onda (de oleaje)  $k$

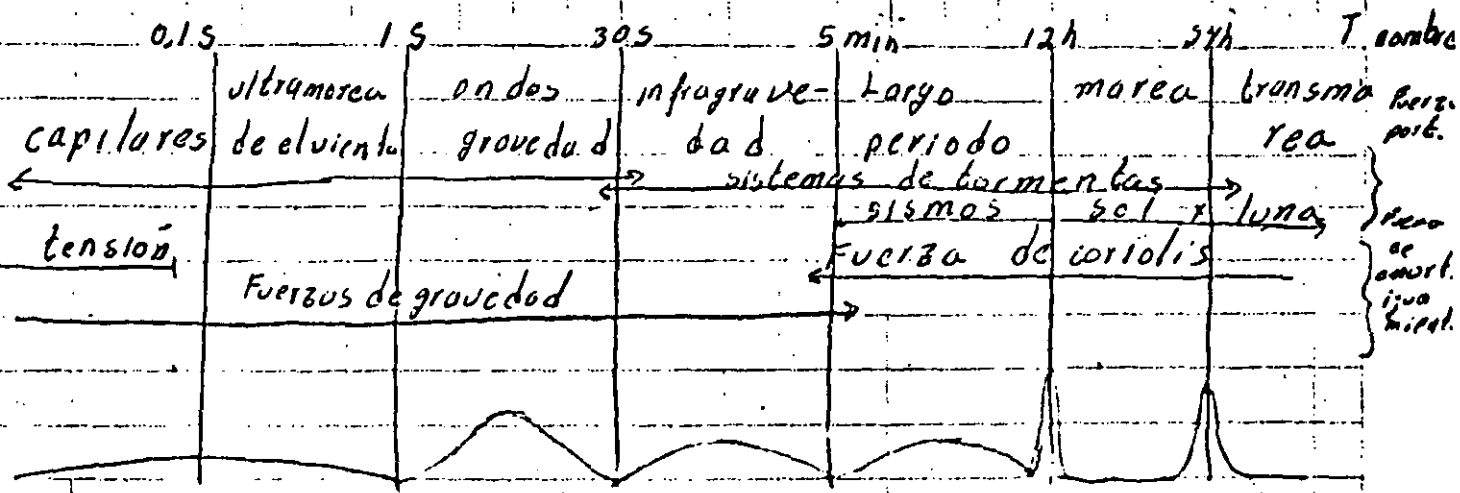
$$k = \frac{2\pi}{L}$$

con lo anterior se tiene que la celeridad también se puede calcular como

$$c = \frac{\omega}{k}$$

### Clasificación de ondas

Munk (1951) según su periodo



## Clasificación del oleaje (continuación)


Características tomadas en cuenta en la clasificación

clasificación

- Altura, longitud y profundidad } ondas de pequeña amplitud (onda senoidal)  
 } ondas de amplitud finita
- Desplazamiento de la cresta } ondas estacionarias (no varían en la dist)  
 } ondas progresivas (varían en la dist y en c/d.)
- profundidad relativa (d/L)

	teoría	práctica
ondas en aguas profundas	$d/L \geq 1$	$d/L \geq 1/2$
ondas en aguas intermedias	$1/20 < d/L < 1$	$1/25 \leq d/L < 1/2$
ondas en aguas bajas	$d/L \leq 1/20$	$d/L \leq 1/25$

- Desplazamiento de las partículas de agua

- oscilatorio 

- translatório 

- Aplicación de la fuerza perturbadora

- libre

- forzada

- Según la teoría de oleaje

- trocoidal

- cnoidal

- solitaria

## 1.1 Ondas de pequeña amplitud.

(teoría lineal, oscilatoria, T. Airy).  
estas ondas tienen la siguiente característica

$$a \ll L ; a \ll d$$

### 1.1.1 Teorema de Bernoulli

se parte de las ecs de Navier-Stokes.

$$\frac{1}{\rho} \frac{\partial p}{\partial x} + \gamma \nabla^2 u + X = \frac{du}{dt} \quad (1.a)$$

$$\frac{1}{\rho} \frac{\partial p}{\partial y} + \gamma \nabla^2 v + Y = \frac{dv}{dt} \quad (1.b)$$

$$\frac{1}{\rho} \frac{\partial p}{\partial z} + \gamma \nabla^2 w + Z = \frac{dw}{dt} \quad (1.c)$$

( $\rho$ : densidad,  $p$ : presión,  $\gamma$ : viscosidad)

en el sistema anterior se tienen tres ecuaciones  
y cuatro incógnitas

Para completar el sistema, se utiliza la  
ecuación de continuidad.

$$\frac{\partial u}{\partial x} + \frac{\partial v}{\partial y} + \frac{\partial w}{\partial z} = 0 \quad (2)$$

## obtención del término $\frac{du}{dt}$

La definición de diferencial total es

$$d(\ ) = \frac{\partial(\ )}{\partial x} dx + \frac{\partial(\ )}{\partial y} dy + \frac{\partial(\ )}{\partial z} dz + \frac{\partial(\ )}{\partial t} dt$$

al dividir entre  $dt$

$$\frac{d(\ )}{dt} = \frac{\partial(\ )}{\partial x} u + \frac{\partial(\ )}{\partial y} v + \frac{\partial(\ )}{\partial z} w + \frac{\partial(\ )}{\partial t}$$

despejando a  $\frac{du}{dt}$

$$\frac{du}{dt} = u \frac{\partial u}{\partial x} + v \frac{\partial u}{\partial y} + w \frac{\partial u}{\partial z} + \frac{\partial u}{\partial t} \quad (3)$$

por otra parte

$$|\vec{V}|^2 = u^2 + v^2 + w^2 = V^2 \quad (4)$$

por lo que

$$\frac{\partial}{\partial x} \left( \frac{V^2}{2} \right) = u \frac{\partial u}{\partial x} + v \frac{\partial v}{\partial x} + w \frac{\partial w}{\partial x}$$

de donde

$$u \frac{\partial u}{\partial x} = \frac{\partial}{\partial x} \left( \frac{V^2}{2} \right) - v \frac{\partial v}{\partial x} - w \frac{\partial w}{\partial x} \quad (5)$$

sust. (5) en (3)

$$\frac{du}{dt} = \frac{\partial}{\partial x} \left( \frac{V^2}{2} \right) - v \frac{\partial v}{\partial x} - w \frac{\partial w}{\partial x} + v \frac{\partial u}{\partial y} + w \frac{\partial u}{\partial z} + \frac{\partial u}{\partial t}$$

$$\frac{du}{dt} = \frac{\partial}{\partial x} \left( \frac{V^2}{2} \right) + v \left( \frac{\partial u}{\partial y} - \frac{\partial v}{\partial x} \right) + w \left( \frac{\partial u}{\partial z} - \frac{\partial w}{\partial x} \right) + \frac{\partial u}{\partial t} \quad (6)$$

por otra parte

$$\text{rot } \hat{V} = \begin{vmatrix} \hat{i} & \hat{j} & \hat{k} \\ \frac{\partial}{\partial x} & \frac{\partial}{\partial y} & \frac{\partial}{\partial z} \\ \mu & \nu & w \end{vmatrix} = \hat{i} \left( \frac{\partial w}{\partial y} - \frac{\partial \nu}{\partial z} \right) - \hat{j} \left( \frac{\partial w}{\partial x} - \frac{\partial \mu}{\partial z} \right) + \hat{k} \left( \frac{\partial \nu}{\partial x} - \frac{\partial \mu}{\partial y} \right)$$

$$[\text{rot } \hat{V}] \times \hat{V} = \begin{vmatrix} \hat{i} & \hat{j} & \hat{k} \\ \left( \frac{\partial w}{\partial y} - \frac{\partial \nu}{\partial z} \right) & \left( \frac{\partial w}{\partial x} - \frac{\partial \mu}{\partial z} \right) & \left( \frac{\partial \nu}{\partial x} - \frac{\partial \mu}{\partial y} \right) \\ \mu & \nu & w \end{vmatrix}$$

La componente del rotacional  $\text{rot } \hat{V}$  según la dirección  $x$  es

$$\underline{\text{rot } \hat{V} \times \hat{V}}_x = w \left( -\frac{\partial w}{\partial x} + \frac{\partial \mu}{\partial z} \right) - \nu \left( \frac{\partial \nu}{\partial x} - \frac{\partial \mu}{\partial y} \right) \quad (7)$$

Al comparar (6) y (7)

$$\frac{d\mu}{dt} = \frac{\partial}{\partial x} \left( \frac{V^2}{2} \right) + \underline{\text{rot } \hat{V} \times \hat{V}}_x + \frac{\partial \mu}{\partial t} \quad (8)$$

sust. (8) en (1.a)

$$\frac{1}{\rho} \frac{\partial p}{\partial x} + \gamma \nabla^2 \mu + X = \frac{\partial \mu}{\partial t} + \frac{\partial}{\partial x} \left( \frac{V^2}{2} \right) + \underline{\text{rot } \hat{V} \times \hat{V}}_x$$



## Hipótesis en la expresión de Bernoulli

$$\frac{1}{\rho} \frac{\partial p}{\partial x} + v \nabla^2 u + X = \frac{\partial u}{\partial t} + \frac{\partial}{\partial x} \left( \frac{v^2}{2} \right) + \text{rot } \vec{v} \times \vec{v} \quad (9)$$

1.-  $v = 0$  ,  $\rho$  es constante

2.- Existe el potencial de velocidad

$$\frac{\partial \phi}{\partial x} = u ; \quad \frac{\partial \phi}{\partial y} = v ; \quad \frac{\partial \phi}{\partial z} = w \quad \text{y} \quad \text{rot } \vec{v} = \vec{0}$$

3.- Existe un potencial de fuerzas másicas  $\Omega = gz$

$$X = \frac{\partial \Omega}{\partial x} = - \frac{\partial}{\partial x} (gz)$$

4.-  $p$  debe ser negativa ya que se trata de "compresión".

$$- \frac{\partial}{\partial x} \left( \frac{p}{\rho} + gz + \frac{v^2}{2} \right) = \frac{\partial u}{\partial t}$$

$$\frac{\partial}{\partial x} \left[ \frac{p}{\rho} + z + \frac{v^2}{2g} \right] = - \frac{1}{g} \frac{\partial u}{\partial t} \quad (10.a)$$

de manera semejante

$$\frac{\partial}{\partial y} \left[ \frac{p}{\rho} + z + \frac{v^2}{2g} \right] = - \frac{1}{g} \frac{\partial v}{\partial t} \quad (10.b)$$

$$\frac{\partial}{\partial z} \left[ \frac{p}{\rho} + z + \frac{v^2}{2g} \right] = - \frac{1}{g} \frac{\partial w}{\partial t} \quad (10.c)$$

Multiplicando por  $dx$ ,  $dy$  y  $dz$ , y considerando

$$\hat{ds} = \hat{i} \cdot dx + \hat{j} \cdot dy + \hat{k} \cdot dz$$

$$d\left(\frac{p}{\gamma} + z + \frac{v^2}{2g}\right) = -\frac{1}{g} \frac{\partial}{\partial t} \hat{V} \cdot \hat{ds} \quad (11)$$

Integrando entre los puntos 1 y 2

$$\left[\frac{p}{\gamma} + z + \frac{v^2}{2g}\right]_1^2 = -\frac{1}{g} \int_1^2 \frac{\partial}{\partial t} \hat{V} \cdot \hat{ds}$$

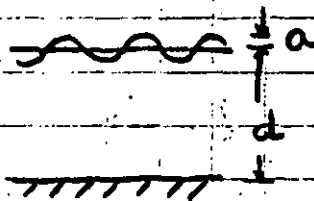
$$\boxed{\left[\frac{p}{\gamma} + z + \frac{v^2}{2g}\right]_1^2 = -\frac{1}{g} \int_1^2 \frac{\partial}{\partial t} \hat{V} \cdot \hat{ds} - \int_1^2 g ds} \quad (12)$$

La expresión (12) es la forma completa de la Teorema de Bernoulli:

Ondas de pequeña amplitud

$$a \ll L$$

$$a \ll d$$



a) Bidimensional  $(x, z)$

$$b) \frac{v^2}{2g} = 0$$

Según la ec. (11)

$$d\left(\frac{p}{\gamma} + z\right) = -\frac{1}{g} \frac{\partial}{\partial t} (u dx + w dz)$$

$$d\left(\frac{p}{\rho} + z\right) = -\frac{1}{\rho} \frac{\partial}{\partial t} \left( \frac{\partial \phi}{\partial x} dx + \frac{\partial \phi}{\partial z} dz \right)$$

$$d\left(\frac{p}{\rho} + z\right) = -\frac{1}{\rho} \frac{\partial}{\partial t} (d\phi)$$

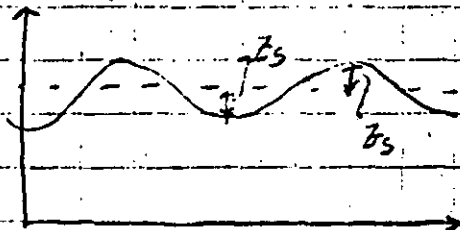
Integrando

$$\frac{p}{\rho} + z = -\frac{1}{\rho} \frac{\partial \phi}{\partial t} + c'$$

$$\frac{p}{\rho} + z + \frac{1}{\rho} \frac{\partial \phi}{\partial t} = c' \quad (13)$$

En la superficie libre  $p$  es constante y que se anula con  $c'$

$$z_s + \frac{1}{\rho} \left( \frac{\partial \phi}{\partial t} \right)_{z=z_s} = 0 \quad (14)$$



$$\eta = z_s$$

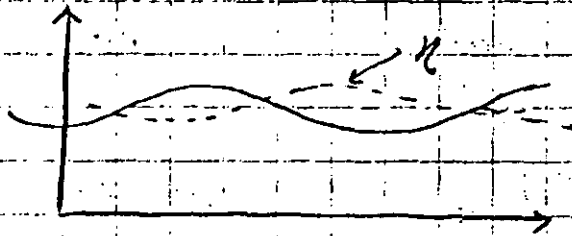
a)-

$$\eta + \frac{1}{\rho} \left( \frac{\partial \phi}{\partial t} \right)_{z=\eta} = 0 \quad (15)$$

b). condición de frontera

La componente vertical de la velocidad ( $w$ ) en la superficie es igual al cambio en el tiempo de esta superficie

$$\frac{d}{dt} \eta(x, t) = W \quad \text{en } z = \eta \quad (16)$$



$$\frac{d}{dt} \eta(x, t) = \frac{\partial \eta}{\partial x} \frac{dx}{dt} + \frac{\partial \eta}{\partial t} = W \quad \text{en } z = \eta$$

$$\frac{d}{dt} \eta = \frac{\partial \eta}{\partial x} \frac{\partial \phi}{\partial x} + \frac{\partial \eta}{\partial t} = \frac{\partial \phi}{\partial z} \quad \text{en } z = \eta \quad (17)$$

$$* \frac{\partial \eta}{\partial x} \frac{\partial \phi}{\partial x} = 0$$

\* serie de Taylor

$$f(x + \Delta x) = f(x) + f'(x) \Delta x + \frac{f''(x)}{2!} \Delta x^2 + \dots + \frac{f^{(n)}(x)}{n!} \Delta x^n + \dots$$

$$\left( \frac{\partial \phi}{\partial z} \right)_{z=\eta} = \left( \frac{\partial \phi}{\partial z} \right)_{z=0} + \left( \frac{\partial^2 \phi}{\partial z^2} \right)_{z=0} \Delta z + \frac{\partial^3 \phi}{\partial z^3} \frac{\Delta z^2}{2} + \dots$$

o sea

$$\left( \frac{\partial \phi}{\partial z} \right)_{z=\eta} = \left( \frac{\partial \phi}{\partial z} \right)_{z=0} \quad (18)$$

del mismo modo

$$y \quad \left( \frac{\partial \phi}{\partial x} \right)_{z=\eta} = \left( \frac{\partial \phi}{\partial x} \right)_{z=0}$$

$$b) \frac{\partial \eta}{\partial t} = \left( \frac{\partial \phi}{\partial z} \right)_{z=0} \quad (19)$$

c) Derivando 15 parcialmente respecto al tiempo e igualando con 16

$$\frac{\partial \eta}{\partial t} + \frac{1}{g} \frac{\partial^2 \phi}{\partial t^2} = 0 \quad \text{en } z = \eta$$

$$\frac{\partial \phi}{\partial z} = -\frac{1}{g} \frac{\partial^2 \phi}{\partial t^2} \quad (20)$$

d) Como la velocidad es cíclica con respecto a la posición horizontal y el tiempo.

$$\phi = f(z) \sin(kx - \tau t)$$

e) Suponiendo  $\eta = a \cos(kx - \tau t)$

f) Ecuación de continuidad

$$\frac{\partial u}{\partial x} + \frac{\partial w}{\partial z} = 0 \quad \frac{\partial^2 \phi}{\partial x^2} + \frac{\partial^2 \phi}{\partial z^2} = 0 \quad (23)$$

g) condición de frontera en el fondo

$$w_{z=d} = \left( \frac{\partial \phi}{\partial z} \right)_{z=d} = 0 \quad (24)$$

★ Determinación de  $f(z)$  :

sust (21) en (23)

$$\frac{\partial \phi}{\partial x} = f(z) k \cos(kx - \tau t)$$

$$\frac{\partial^2 \phi}{\partial x^2} = -f(z) k^2 \sin(kx - \tau t)$$

$$\frac{\partial \phi}{\partial z} = f'(z) \sin(kx - \tau t)$$

$$\frac{\partial^2 \phi}{\partial z^2} = f''(z) \sin(kx - \tau t)$$

$$-f(z) k^2 \sin(kx - \tau t) + f''(z) \sin(kx - \tau t) = 0$$

$$f''(z) - k^2 f(z) = 0 \quad ; \quad f(z) = A e^{kz} + B e^{-kz} \quad (26)$$

$$\phi = (A e^{kz} + B e^{-kz}) \sin(kx - \tau t) \quad (27)$$

procedimiento para valorar A y B (27) en (24)

$$\frac{\partial \phi}{\partial z} = (A k e^{kz} - B k e^{-kz}) \sin(kx - \tau t)$$

$$(A k e^{kd} - B k e^{-kd}) \sin(kx - \tau t) = 0$$

$$A e^{-kd} - B e^{kd} = 0 \quad (28)$$

$$\text{si } Ae^{-kd} = Be^{kd} = D/2$$

$$A = \frac{D}{2} e^{kd} \quad B = \frac{D}{2} e^{-kd}$$

subst (27)

$$\phi = \frac{D}{2} \left[ e^{k(d+z)} + e^{-k(d+z)} \right] \text{sen}(kx - \tau t)$$

$$\phi = D \cosh k(z+d) \text{sen}(kx - \tau t)$$

Segun 19

$$\eta = -\frac{1}{g} \left( \frac{\partial \phi}{\partial t} \right)_{z=0}$$

$$\frac{\partial \phi}{\partial t} = \tau D \cosh k(z+d) \cos(kx - \tau t)$$

$$\left( \frac{\partial \phi}{\partial t} \right)_{z=0} = -\tau D \cosh kd \cos(kx - \tau t)$$

$$\eta = \frac{1}{g} \tau D \cosh kd \cos(kx - \tau t)$$

al comparar con 22

$$\eta = a \cos(kx - \tau t)$$

$$a = \frac{1}{g} \sqrt{D} \cosh kd \quad ; \quad D = \frac{ag}{\sqrt{\cosh kd}}$$

Así

$$\phi = \frac{ag \cosh k(z+d) \operatorname{sen}(kx - \sqrt{g}t)}{\cosh kd} \quad (32)$$



Sea la oc.

Otros resultados de importancia

$$\phi = (Ae^{kz} + Be^{-kz}) \operatorname{sen}(kx - \sqrt{g}t) \quad (27)$$

$$\frac{\partial \phi}{\partial t} = (Ae^{kz} + Be^{-kz}) \cos(kx - \sqrt{g}t) (-\sqrt{g})$$

$$\frac{\partial^2 \phi}{\partial t^2} = -\sqrt{g}^2 (Ae^{kz} + Be^{-kz}) \operatorname{sen}(kx - \sqrt{g}t)$$

en  $z=0$

$$\left( \frac{\partial^2 \phi}{\partial t^2} \right)_{z=0} = -\sqrt{g}^2 (A+B) \operatorname{sen}(kx - \sqrt{g}t)$$

$$\left( \frac{\partial \phi}{\partial z} \right)_{z=0} = k(A-B) \operatorname{sen}(kx - \sqrt{g}t)$$

Según 20

$$\frac{\partial \phi}{\partial z} = -\frac{1}{g} \frac{\partial^2 \phi}{\partial t^2}$$



$$k(A-B) \operatorname{sen}(kx - vt) = \frac{v}{g} v^2 (A+B) \operatorname{sen}(kx - vt)$$

$$v^2 (A+B) = +kg (A-B)$$

$$(v^2 - kg)A + (v^2 + kg)B = 0$$

La 2ª es

$$Ae^{-kd} - Be^{kd} = 0$$

Lo anterior es un sistema de ec. lineal y homogéneo.

$$\begin{vmatrix} v^2 - kg & v^2 + kg \\ e^{-kd} & -e^{kd} \end{vmatrix} = (v^2 - kg)e^{kd} + (v^2 + kg)e^{-kd} = 0$$

$$v^2 e^{kd} - kg e^{kd} + v^2 e^{-kd} + kg e^{-kd} = 0$$

$$v^2 (e^{kd} + e^{-kd}) - kg (e^{kd} - e^{-kd}) = 0$$

$$v^2 (2 \operatorname{cosh} kd) - kg (2 \operatorname{senh} kd) = 0$$

$$v^2 = kg \frac{\operatorname{senh} kd}{\operatorname{cosh} kd}$$

$$v^2 = kg \operatorname{tan} h kd$$

recordando que

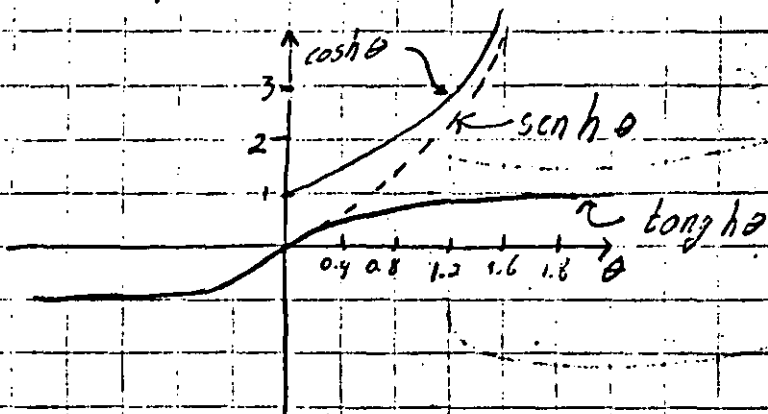
$$\sigma = \frac{2\pi}{T} \quad \text{y} \quad k = \frac{2\pi}{L}$$

$$\left(\frac{2\pi}{T}\right)^2 = \frac{2\pi}{L} g \tanh \frac{2\pi d}{L} \quad ; \quad \frac{L}{T} = \frac{T}{2\pi} g \tanh \frac{2\pi d}{L}$$

$$C = \frac{g T}{2\pi} \tanh \frac{2\pi d}{L}$$

$$C = \sqrt{\frac{g L}{2\pi}} \tanh \frac{2\pi d}{L}$$

Gráfica de las funciones hiperbólicas



$$\text{si } \frac{d}{L} \rightarrow \infty$$

$$\Rightarrow \tanh \frac{2\pi d}{L} \rightarrow 1$$

El subíndice 0 indica condición de aguas profundas

$$C_0 = \frac{g T}{2\pi}$$

$$C_0 = \sqrt{\frac{g L}{2\pi}}$$

$$L_0 = \frac{g T^2}{2\pi} = 1.56 T^2$$

(aproximaciones)

$$\text{si } \theta = 0 \quad \tanh \theta = 0 \quad \frac{d}{L} \rightarrow 0$$

$$\text{si } \frac{d}{L} \leq \frac{1}{25} \quad \tanh \frac{2\pi d}{L} \approx \frac{2\pi d}{L}$$

$$c = \frac{gT}{2\pi} \frac{2\pi d}{L}$$

$$c = \frac{gT d}{L}$$

$$c \frac{L}{T} = g d$$

$$c = g d$$

$$c = \sqrt{g d}$$

aguas bajas

$$L = T \sqrt{g d}$$

aguas bajas

tambien se deduce

$$L = \frac{gT^2}{2\pi} \frac{\tanh \frac{2\pi d}{L}}{L}$$

La expresion anterior se puede resolver por aproximaciones sucesivas o utilizando las tablas de Wiegel.

\* características del oleaje  
 recordando que  $\phi$

$$\phi = \frac{a g}{\sqrt{\cos h b d}} \frac{\cosh k(z+d)}{\cosh k d} \sin h (kx - \tau t)$$

- La velocidad horizontal es

$$u = \frac{\partial \phi}{\partial x} = \frac{a g k \cosh k(z+d) \cos(kx - \tau t)}{\tau \cosh kd}$$

- La velocidad vertical es

$$w = \frac{\partial \phi}{\partial z} = \frac{a g k \sinh k(z+d) \sin(kx - \tau t)}{\tau \cosh kd}$$

de otra manera también

$$u = \frac{a g k \sinh kd \cosh k(z+d) \cos(kx - \tau t)}{\tau \sinh kd \cosh kd}$$

y como  $c = \frac{g}{\tau} \tanh kd$  ;  $\tanh kd = \frac{c\tau}{g}$

$$u = a k c \frac{\cosh k(z+d) \cos(kx - \tau t)}{\sinh kd}$$

- otra forma alternativa es (el desplazamiento)

$$u = \frac{dx}{dt} ; dx = u dt ; x = \int u dt + X_0$$

desplazamiento

$$x = \frac{a g k \cosh k(z+d)}{\tau \cosh kd} \int \cos(kx - \tau t) dt + X_0$$

$$x = - \frac{a g k \cosh k(z+d) \sin(kx - \tau t)}{\tau^2 \cosh kd} + X_0$$

el desplazamiento vertical es  $z$

$$W = \frac{dz}{dt} \quad z = \int W dt + z_0$$

$$z = \frac{a g k}{V^2} \frac{\sinh k(z+d) \cos(kx - Tt) + z_0}{\cosh kd}$$

$$\sin(kx - Tt) = \frac{X - X_0}{\frac{a g k}{V^2} \frac{\cosh k(z+d)}{\cosh kd}}$$

$$\cos(kx - Tt) = \frac{z - z_0}{\frac{a g k}{V^2} \frac{\sinh k(z+d)}{\cosh kd}}$$

como  $\sin^2(kx - Tt) + \cos^2(kx - Tt) = 1$

$$\left[ \frac{X - X_0}{\frac{a g k}{V^2} \frac{\cosh k(z+d)}{\cosh kd}} \right]^2 + \left[ \frac{z - z_0}{\frac{a g k}{V^2} \frac{\sinh k(z+d)}{\cosh kd}} \right]^2 = 1$$

- esta es la ec. de una elipse pero en aguas profundas las trayectorias son circulares, y en aguas intermedias tienen forma de una elipse.  
de manera semejante se encuentran

$$dx = \frac{dW}{dt}$$

$$dz = \frac{dW}{dt}$$

$p$  (presión),  $E$  (energía),  $\bar{P}$  (potencia media)

• Cálculo de la presión  $p$ .

si  $c=0$  en la ec. 13

$$\frac{p}{\gamma} + z + \frac{1}{g} \frac{\partial \phi}{\partial t} = 0 \quad (13')$$

$$\text{como } \frac{\partial \phi}{\partial t} = \frac{\partial}{\partial t} \left\{ \frac{ag}{\gamma} \frac{\cosh k(z+d) \sin(kx - \tau t)}{\cosh kd} \right\}$$

$$\frac{p}{\gamma} + z - a \frac{\cosh k(z+d) \cos(kx - \tau t)}{\cosh kd} = 0$$

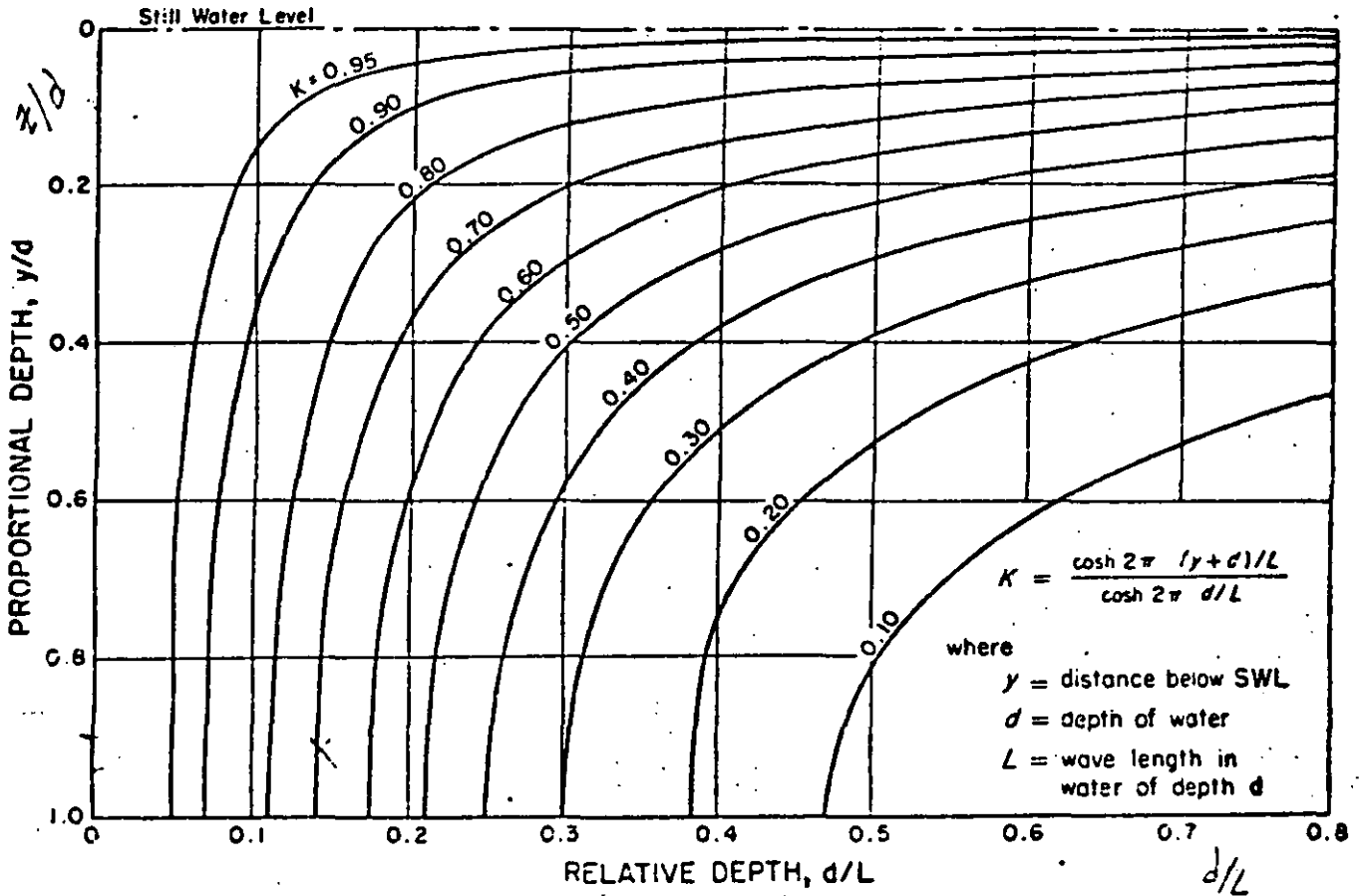
$$\text{como } \eta = a \cos(kx - \tau t)$$

$$\frac{p}{\gamma} + z = \eta \frac{\cosh k(z+d)}{\cosh kd}$$

$k$  es el fac de respuesta

$$k = \frac{\cosh k(z+d)}{\cosh kd}$$

Fig. 2.8. Pressure response factor  $K$ , linear theory (after Wiegell and Johnson, 1951)



## ★ Celeridad de un tren de ondas.

$$\eta = a \cos(kx - \omega t)$$

$$c = \frac{L}{T} = \frac{\frac{1}{2}\pi}{\frac{1}{2}\pi} = \frac{\omega}{k} \quad \omega = ck$$

$$\eta = a \cos[k(x - ct)]$$

si  $\eta = \eta_1 + \eta_2$

$$\eta = a \cos k_1(x + c_1 t) + a \cos k_2(x + c_2 t)$$

por otra parte

$$\cos A + \cos B = 2 \cos\left(\frac{A+B}{2}\right) \cos\left(\frac{A-B}{2}\right)$$

$$\eta = 2a \cos\left[\frac{(k_1 x + c_1 t) + (k_2 x + c_2 t)}{2}\right] \cos\left[\frac{(k_1 x + c_1 t) - (k_2 x + c_2 t)}{2}\right]$$

$$\eta = 2a \cos\left[\frac{k_1 + k_2}{2}x - \frac{c_1 t + c_2 t}{2}\right] \cos\left[\frac{k_1 - k_2}{2}x - \frac{(c_1 - c_2)t}{2}\right]$$

análisis del primer argumento:

número de onda  $\frac{k_1 + k_2}{2}$

frecuencia angular  $\frac{k_1 c_1 + k_2 c_2}{2}$

longitud de onda  $\frac{2\pi}{k_1 + k_2} = \frac{4\pi}{k_1 + k_2}$



$$\text{periodo} = \frac{4\pi}{k_1 c_1 + k_2 c_2}$$

análisis del segundo argumento

$$\text{número de onda} = \frac{k_1 - k_2}{2}$$

$$\text{frecuencia angular} = \frac{k_1 c_1 - k_2 c_2}{2}$$

$$\text{longitud de onda} = \frac{4\pi}{k_1 - k_2}$$

$$\text{periodo} = \frac{4\pi}{k_1 c_1 - k_2 c_2}$$

Cuando  $k_1 \doteq k_2 = k$  y  $c_1 \doteq c_2 = c$

$$1^{\circ} \text{ Arg. } L = \frac{2\pi}{k} \quad T = \frac{2\pi}{kc}$$

representa las características de una sola onda

2° Arg.  $L$  y  $T$  tienden a infinito

$$C_g = \frac{\frac{4\pi}{k_1 c_1 - k_2 c_2}}{\frac{4\pi}{k_1 - k_2}} = \frac{k_2 c_1 - k_1 c_2}{k_1 - k_2} = \frac{\Delta kc}{\Delta k}$$

$$\text{si } \Delta k \rightarrow 0 \quad C_g = \frac{d(kc)}{dk}$$

$$C_g = k \frac{dc}{dk} + c \frac{dk}{dk} = k \frac{dc}{dk} + c$$

recordando que

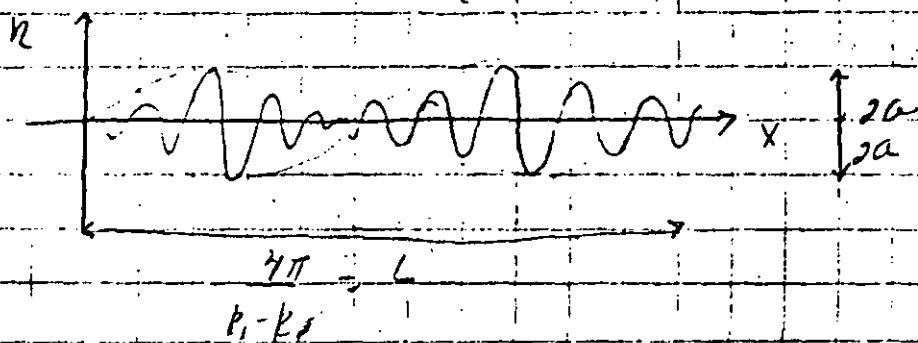
$$c = \sqrt{\frac{g \tanh kd'}{k}}$$

con lo anterior se obtiene

$$c_g = \frac{1}{2} \left[ 1 + \frac{4\pi d}{L} \frac{1}{\operatorname{sen} h 4\pi d/L} \right] c$$

ó  $c_g = n c$

$c_g$  corresponde a la velocidad de propagación de la envolvente de un tren de ondas.



En aguas profundas  $n_0 = 0.5$

En aguas bajas  $n = 1$

a  $n$  también se le llama relación de velocidades.

Tarea: Hacer un resumen sobre teoría del oleaje de amplitud finita. Máximo dos hojas tamaño carta

TABLA 2.1 Compendio de ecuaciones de la teoría lineal

Características de onda	Número de eq. del capítulo A	Ecuación general (aguas intermedias)	Aguas profundas ( $d/L > 1/2$ )	Aguas poco profundas ( $d/L < 1/25$ )
Forma de la superficie ( $\eta$ )	A.3.22	$\eta = a \cos(kx - \omega t) = a \cos \theta$		
Velocidad ( $C = L/T$ )	A.3.19	$C = \frac{g}{k} \tanh kd$	$C = \sqrt{\frac{gL}{2\pi}}$	$C = \sqrt{gd}$
Longitud ( $L$ )	A.3.21	$L = \frac{gT^2}{2\pi} \tanh kd$	$L = \frac{gT^2}{2\pi} = 1.52T^2$	$L = T\sqrt{gd}$
Velocidad de grupo ( $C_g$ )	A.3.47	$C_g = nC = \frac{C}{2} \left[ 1 + \frac{2kd}{\sinh 2kd} \right]$	$C_g = \frac{C}{2}$	$C_g = \sqrt{gd}$
Velocidad horizontal de la partícula ( $u$ )	A.3.11	$u = \frac{kga}{\sigma} \frac{\cosh[k(z+d)]}{\cosh kd} \cos \theta$	$u = \frac{a}{\sigma} e^{kz} \cos \theta$	$u = a \sqrt{\frac{g}{d}} \cos \theta$
Velocidad vertical de la partícula ( $w$ )	A.3.13	$w = \frac{kga}{\sigma} \frac{\sinh[k(z+d)]}{\cosh kd} \sin \theta$	$w = \frac{a}{\sigma} e^{kz} \sin \theta$	$w = \frac{2a}{T} \pi \left( 1 + \frac{z}{d} \right) \sin \theta$
Aceleración horizontal de la partícula ( $a_x$ )	A.3.15a	$a_x = kga \frac{\cosh[k(z+d)]}{\cosh kd} \sin \theta$	$a_x = a\sigma^2 e^{kz} \sin \theta$	$a_x = a\sigma \sqrt{\frac{g}{d}} \sin \theta$
Aceleración vertical de la partícula ( $a_z$ )	A.3.15b	$a_z = -kga \frac{\sinh[k(z+d)]}{\cosh kd} \cos \theta$	$a_z = -a\sigma^2 e^{kz} \cos \theta$	$a_z = -a\sigma^2 \left( 1 + \frac{z}{d} \right) \cos \theta$
Desplazamiento horizontal de la partícula ( $\xi$ )	A.3.28	$\xi = -a \frac{\cosh[k(z+d)]}{\sinh kd} \sin \theta$	$\xi = -a e^{kz} \sin \theta$	$\xi = -\frac{a}{\sigma} \sqrt{\frac{g}{d}} \sin \theta$
Desplazamiento vertical de la partícula ( $\zeta$ )	A.3.29	$\zeta = a \frac{\sinh[k(z+d)]}{\sinh kd} \cos \theta$	$\zeta = a e^{kz} \cos \theta$	$\zeta = a \left( 1 + \frac{z}{d} \right) \cos \theta$
Presión abajo de la superficie ( $P$ )	A.3.33	$P = \gamma \eta \frac{\cosh[k(z+d)]}{\cosh kd} - \gamma z$	$P = \gamma \eta e^{kz} - \gamma z$	$P = \gamma (\eta - z)$
Energía total por unidad de masa ( $E_T$ )	A.3.40	$E = \frac{\gamma a^2}{2} L$	$E = \frac{\gamma g a^2 T^2}{2\pi}$	$E = \frac{\gamma a^2}{2} T\sqrt{gd}$
Potencia por unidad de frente por unidad de ancho ( $\bar{P}$ )	A.3.48	$\bar{P} = \frac{nE}{T}$	$\bar{P} = \frac{\gamma g T a^2}{8\pi}$	$\bar{P} = \frac{\gamma a^2}{2} \sqrt{gd}$
Potencial de velocidad ( $\phi$ )	A.3.10	$\phi = \frac{ga}{\sigma} \frac{\cosh k(z+d)}{\cosh kd} \sin \theta$	$\phi = \frac{ga}{\sigma} e^{kz} \sin \theta$	$\phi = \frac{ga}{\sigma} \sin \theta$

Compendio de ecuaciones de la teoría lineal.

# ★ Ondas Capilares

Cuando  $L$  se reduce y es muy pequeña empieza a tener importancia la tensión superficial.

$$\left(\frac{\partial \phi}{\partial t}\right)_{z=0} + g\eta + \frac{K}{\rho} \frac{\partial^2 \eta}{\partial x^2} = 0$$

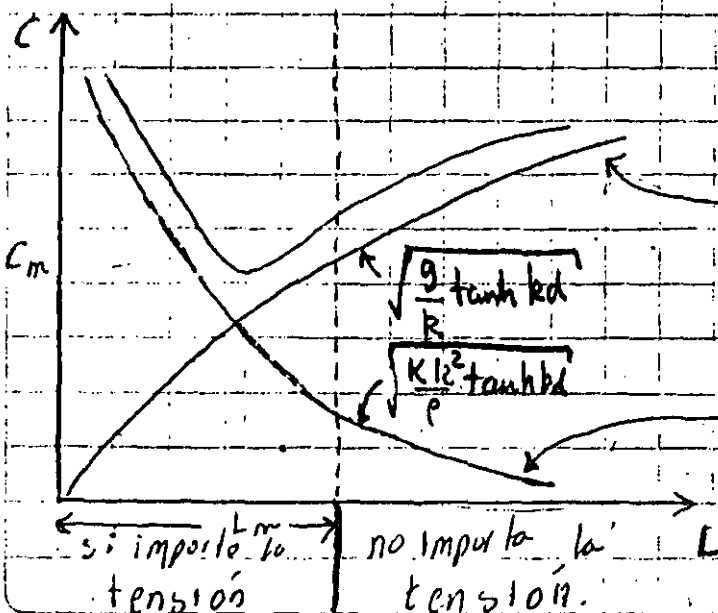
$K$  es función la temperatura

$$\eta = a \cos(kx - \pi t)$$

$$\frac{\partial \eta}{\partial x} = -ka \sin(kx - \pi t); \quad \frac{\partial^2 \eta}{\partial x^2} = -k^2 a \cos(kx - \pi t) = -k^2 \eta$$

$$\left(\frac{\partial \phi}{\partial t}\right)_{t=0} + \left(g - \frac{K}{\rho} k^2\right) \eta = 0$$

$$C = \sqrt{\left(\frac{g}{k} + \frac{K}{\rho} k^2\right) \tanh kd}$$



( $L$  grande)

$$C = \sqrt{\frac{g}{k}} = \sqrt{\frac{gL'}{2\pi}}$$

( $L$  grande)

$$C = \sqrt{\frac{K k^2}{\rho}} = \sqrt{\frac{K'}{\rho} \frac{2\pi}{L}}$$

de la figura anterior

$$K = 73.5 \text{ dinas/cm} \quad \text{a } 15^\circ\text{C}$$

$$C_m = 23.2 \text{ cm/sec}$$

$$L_m = 1.72 \text{ cm}$$

∴ No existen en la naturaleza ondas con una celeridad menor a 23.5 cm/sec

## 1.2 ONDAS DE AMPLITUD FINITA

recordando que

$$\eta = -\frac{1}{g} \left( \frac{\partial \phi}{\partial t} \right)_{z=\eta} \quad (15)$$

$$\frac{\partial \phi}{\partial z} + \frac{1}{g} \frac{\partial^2 \phi}{\partial t^2} = 0 \quad \text{en } z = \eta \quad (16)$$

$$\nabla^2 \phi = 0 \quad (17) \quad \nabla^2 \eta = 0$$

Tomando en cuenta que  $\nabla^2 \eta = 0$

$$\hat{V} = u \hat{i} + v \hat{j} + w \hat{k}$$

$$d \left( \frac{x}{g} + z + \frac{v^2}{2g} \right) = -\frac{1}{g} \frac{\partial \hat{V} \cdot d\hat{s}}{\partial t} \quad (11)$$

$$\hat{V} = \frac{\partial \phi}{\partial x} \hat{i} + \frac{\partial \phi}{\partial y} \hat{j} + \frac{\partial \phi}{\partial z} \hat{k} \quad ; \quad \hat{V} = \hat{\nabla} \phi$$

$$V^2 = |\vec{V}| = \vec{V} \cdot \vec{V} = \hat{\nabla} \phi \cdot \hat{\nabla} \phi$$

$$\vec{V} \cdot d\vec{s} = \hat{\nabla} \phi \cdot d\vec{s}$$

$$\left( \frac{\partial \phi}{\partial x} \hat{i} + \frac{\partial \phi}{\partial y} \hat{j} + \frac{\partial \phi}{\partial z} \hat{k} \right) \cdot (dx \hat{i} + dy \hat{j} + dz \hat{k})$$

$$\vec{V} \cdot d\vec{s} = d\phi$$

$$d \left( \frac{p}{\rho} + z + \frac{1}{2g} \hat{\nabla} \phi \cdot \hat{\nabla} \phi \right) = \frac{1}{g} \frac{\partial \phi}{\partial t} dt$$

$$= d \left( \frac{1}{g} \frac{\partial \phi}{\partial t} \right)$$

$$d \left( \frac{p}{\rho} + z + \frac{1}{2g} \hat{\nabla} \phi \cdot \hat{\nabla} \phi + \frac{1}{g} \frac{\partial \phi}{\partial t} \right) = 0 \quad (34)$$

Lo anterior es el teorema de Bernoulli para la 2ª Aprox (caso en que  $V^2/2g \neq 0$ )

$$\frac{p}{\rho} + z + \frac{1}{2g} \hat{\nabla} \phi \cdot \hat{\nabla} \phi + \frac{1}{g} \frac{\partial \phi}{\partial t} = C, \quad (34')$$

$$p = 0 \quad \text{en} \quad z = \eta$$

$$\eta = - \frac{1}{g} \left( \frac{\partial \phi}{\partial t} + \frac{1}{2} \hat{\nabla} \phi \cdot \hat{\nabla} \phi \right)_{z=\eta} + C, \quad (36)$$

se usa en lugar de B15

C. De varias identidades matematicas

$$\frac{dP}{dt} = \frac{\partial P}{\partial x} \left( \frac{dx}{dt} \right) + \frac{\partial P}{\partial y} \left( \frac{dy}{dt} \right) + \frac{\partial P}{\partial z} \left( \frac{dz}{dt} \right) + \frac{\partial P}{\partial t} \frac{dt}{dt}$$

$\quad \quad \quad u \quad \quad \quad v \quad \quad \quad w \quad \quad \quad 1$

$$\frac{1}{\rho} \frac{dP}{dt} = \frac{1}{\rho} \left[ \frac{\partial P}{\partial x} u + \frac{\partial P}{\partial y} v + \frac{\partial P}{\partial z} w + \frac{\partial P}{\partial t} \right]$$

$$\frac{d}{dt} \left( \frac{P}{\rho} \right) = \frac{\partial (P/\rho)}{\partial x} u + \frac{\partial (P/\rho)}{\partial y} v + \frac{\partial (P/\rho)}{\partial z} w + \frac{\partial (P/\rho)}{\partial t}$$

$$= \frac{\partial \phi}{\partial x} \frac{\partial (P/\rho)}{\partial x} + \frac{\partial \phi}{\partial y} \frac{\partial (P/\rho)}{\partial y} + \frac{\partial \phi}{\partial z} \frac{\partial (P/\rho)}{\partial z} + \frac{\partial (P/\rho)}{\partial t}$$

$$= \hat{\nabla} \phi \cdot \hat{\nabla} (P/\rho) + \frac{\partial}{\partial t} \left( \frac{P}{\rho} \right)$$

$$\frac{d}{dt} \left( \frac{P}{\rho} \right) = \left[ \hat{\nabla} \phi \cdot \hat{\nabla} + \frac{\partial}{\partial t} \right] \left( \frac{P}{\rho} \right) \quad (36)$$

multiplicando 34 por  $g$  y ordenando

$$\frac{P}{\rho} = - \left( z g + \frac{1}{2} \hat{\nabla} \phi \cdot \hat{\nabla} \phi + \frac{\partial \phi}{\partial t} \right) + C_2 \quad (37)$$

sustituyendo 37 en 36

$$\left[ \hat{\nabla} \phi \cdot \hat{\nabla} + \frac{\partial}{\partial t} \right] \left[ - z g - \frac{1}{2} \hat{\nabla} \phi \cdot \hat{\nabla} \phi - \frac{\partial \phi}{\partial t} + C_2 \right] = \frac{d}{dt} \left( \frac{P}{\rho} \right)$$

$$g \left[ \frac{\hat{\nabla} \phi \cdot \hat{\nabla}}{\partial t} + 2 \right] z + \frac{1}{2} \left[ \frac{\hat{\nabla} \phi \cdot \hat{\nabla}}{\partial t} + 2 \right] (\hat{\nabla} \phi \cdot \hat{\nabla} \phi) +$$

$$+ \left[ \frac{\hat{\nabla} \phi \cdot \hat{\nabla}}{\partial t} + 2 \right] \frac{\partial \phi}{\partial t} = - \frac{d}{dt} \left( \frac{p}{\rho} \right)$$

$$g \left[ \frac{\partial \phi}{\partial x} \frac{\partial}{\partial x} + \frac{\partial \phi}{\partial z} \frac{\partial}{\partial z} + 2 \right] z + \frac{1}{2} \left[ \frac{\nabla \phi \cdot \nabla}{\partial t} + 2 \right] (\nabla \phi \cdot \nabla \phi) +$$

$$\hat{\nabla} \phi \cdot \hat{\nabla} \left( \frac{\partial \phi}{\partial t} \right) + \frac{\partial^2 \phi}{\partial t^2} = - \frac{d}{dt} \left( \frac{p}{\rho} \right)$$

$$g \frac{\partial \phi}{\partial z} + g \frac{\partial z}{\partial t} + \frac{\partial^2 \phi}{\partial t^2} + \frac{1}{2} \left[ \frac{\hat{\nabla} \phi \cdot \hat{\nabla}}{\partial t} + 2 \right] (\hat{\nabla} \phi \cdot \hat{\nabla} \phi + \hat{\nabla} \phi \cdot \hat{\nabla} \left( \frac{\partial \phi}{\partial t} \right)) =$$

$$= \frac{d}{dt} \left( \frac{p}{\rho} \right)$$

$$\text{en } z=0, \quad p=0, \quad \frac{\partial p}{\partial t} = 0, \quad \frac{\partial z}{\partial t} = 0$$

$$\left( g \frac{\partial \phi}{\partial z} + \frac{\partial^2 \phi}{\partial t^2} \right) + \hat{\nabla} \phi \cdot \hat{\nabla} \left( \frac{\partial \phi}{\partial t} \right) + \frac{1}{2} \left[ \frac{\hat{\nabla} \phi \cdot \hat{\nabla}}{\partial t} + 2 \right] (\hat{\nabla} \phi \cdot \hat{\nabla} \phi) = 0$$

Terminos de: 1° Orden      2° Orden      3° Orden

## 1.21. TEORIA DE STOKES

Sea el potencial de velocidades

$$\phi(x, z) = -Cx - C\beta e^{kz} \sin kx$$

y la función línea de corriente

$$\psi(x, z) = -Cz + C\beta e^{kz} \cos kx$$

para  
aguas  
profundas



Proponiendo

$$\eta = \beta e^{kx} \cos kx$$

como

$$e^{\theta} = 1 + \theta + \frac{1}{2!} \theta^2 + \frac{1}{3!} \theta^3 + \frac{1}{4!} \theta^4 + \dots$$

así

$$\eta = \beta \left[ 1 + k\eta + \frac{1}{2} k^2 \eta^2 + \frac{1}{6} k^3 \eta^3 + \dots \right] \cos kx$$

sust. (34) y (42) en (35)

$$\left( \frac{2g}{c^2} - 2k \right) \eta + k^2 \beta^2 e^{2k\eta} = G \quad (45)$$

desarrollando en series  $e^{2k\eta}$ , sust (43), y despreciando terminos de orden 3 en adelante

$$\left( \frac{2g}{c^2} - 2k - 2k^3 \beta^2 \right) \eta + (2k^4 \beta^2) \eta^2 = G$$

$$\frac{2g}{c^2} - 2k + 2k^3 \beta^2 = 0 \quad 2k^4 \beta^2 = G$$

proponiendo

$$\eta = \beta \eta_0 + \beta^2 \eta_1 + \beta^3 \eta_2$$

$$\eta = \beta \eta_0 + \beta^2 \eta_1 + \beta^3 \eta_2 \quad (48)$$

sus. t. en 44

$$\eta = \beta \left[ 1 + k \eta_0 + \frac{1}{2} k^2 \eta_0^2 + \frac{1}{6} k^3 \eta_0^3 + \frac{1}{24} k^4 \eta_0^4 \right] \cos kx$$

$$\begin{aligned} \beta \eta_0 + \beta^2 \eta_1 + \beta^3 \eta_2 &= \beta \left[ 1 + \beta k \eta_0 + \beta^2 k \eta_1 + \beta^3 \left( \frac{1}{2} k^2 \eta_0 \right) \right] \cos kx \\ &= \beta \left[ 1 + \beta k \eta_0 + \beta^2 k \eta_1 + \beta^3 \left( \frac{1}{2} k^2 \eta_0 \right) \right] \cos kx \end{aligned}$$

igualando los coeficientes

$$\eta_0 = \cos kx$$

$$\eta_1 = k \eta_0 \cos kx = k \cos^2 kx = \frac{1}{2} k + \frac{1}{2} k \cos 2kx$$

$$\eta_2 = (k \eta_1 + \frac{1}{2} k^2 \eta_0) \cos kx$$

$$\eta_2 = \frac{9}{8} k^2 (\cos kx + \frac{3}{8} \cos 3kx)$$

$$\eta = \beta \cos kx + \beta^2 \frac{k}{2} (1 + \cos 2kx) + \frac{9}{8} \beta^3 k^2 (\cos kx + \frac{3}{8} \cos 3kx)$$

$$\eta = \frac{1}{2} k \beta^2 + \beta \underbrace{\left( 1 + \frac{9}{8} k^2 \beta^2 \right)}_a \cos kx + \frac{1}{2} k \beta^2 \cos 2kx + \frac{3}{8} k^2 \beta^3 \cos 3kx$$

se propone

$$\beta = \epsilon_1 a + \epsilon_2 a^2 + \epsilon_3 a^3$$

$$\epsilon_1 = ? \quad ; \quad \epsilon_2 = ? \quad ; \quad \epsilon_3 = ?$$

$$a = \beta \left(1 + \frac{g}{8} B^2 k^2\right)$$

$$a = (\epsilon_1 a + \epsilon_2 a^2 + \epsilon_3 a^3) \left(1 + \frac{g}{8} [\epsilon_1 a + \epsilon_2 a^2 + \epsilon_3 a^3] k^2\right)$$

$$a = a \epsilon_1 + a^2 \epsilon_2 + a^3 (\epsilon_3 + \frac{g}{8} k^2 \epsilon_1) + \dots$$

$$1 = \epsilon_1 \quad ; \quad 0 = \epsilon_2 \quad ; \quad 0 = \epsilon_3 + \frac{g}{8} k^2 \epsilon_1 = \epsilon_3 + \frac{g}{8} k^2$$

$$\beta = 1a + 0a^2 + \left(-\frac{g}{8} k^2\right) a^3$$

$$\beta = a$$

$$\eta = a \cos kx + \frac{a^2 k}{8} (1 + \cos 2kx) + \frac{g}{8} a^3 k^2 \left(\cos kx + \frac{3}{8} \cos^3 kx\right)$$

esta es la ecuación de la superficie libre según la 3ª aproximación de STOKES (aguas profundas)

$$\left[ \eta = a \cos kx \quad (\text{teoría lineal}) \right]$$

$$\frac{2g}{c^2} - 2k + 2k^3 B^2 = 0 \quad (46)$$

mult. por  $c^2/2k$

$$\frac{g}{k} - c^2 + c^2 k^2 B^2 = 0 \quad ; \quad c^2 = \frac{g}{k} + c^2 k^2 B^2$$

se propone

$$c^2 = b_0 + b_1 B + b_2 B^2 + b_3 B^3 \quad b_i = ?$$

$$b_0 + b_1 B + b_2 B^2 + b_3 B^3 = \frac{g}{k} + (b_0 + b_1 B + b_2 B^2 + b_3 B^3) k^2 B^2$$

$$b_0 = \frac{g}{k} ; b_1 = 0 ; b_2 = k^2 b_0 = k^2 \frac{g}{k} = kg$$

$$b_3 = 0 ; c^2 = \frac{g}{k} + 0B + kg B^2 + 0B^3$$

$$c^2 = \frac{g}{k} (1 + k^2 B^2) \quad \left( c^2 = \frac{g}{k} (1 + k^2 a^2) \right)$$

esta es la ecuación de la celeridad en aguas profundas según la tercera aproximación de STOKES.

Para la 1ª Aprox. (en aguas profundas)  $c^2 = g/k$

STOKES 2ª Aprox.

$$\eta = a \cos(kx - \omega t) + \frac{\pi H^2}{8L} \frac{\cos[2\pi \frac{x}{L} (2 + \cosh 2\pi d/L)]}{\sinh^3(2\pi d/L)} \cos^2(kx - \omega t)$$

$$c = \frac{gT}{2\pi} \frac{\tanh 2\pi d}{L} \quad ; \quad L = \frac{gT^2}{2\pi} \frac{\tanh 2\pi d}{L}$$

### 3° Aproximación - STOKES.

$$\eta = a \cos(kx - \sigma t) + \frac{\pi^2 a^2}{L} f_2\left(\frac{d}{L}\right) \cos 2(kx - \sigma t) + \frac{\pi^2 a^3}{L^2} f_3\left(\frac{d}{L}\right) \cos 3(kx - \sigma t)$$

$$f_2\left(\frac{d}{L}\right) = \frac{(2 + \cos 2kd) \cosh kd}{25 \operatorname{senh}^2 kd}$$

$$f_3\left(\frac{d}{L}\right) = \frac{3}{16} \frac{1 + 9 \cosh^6 kd}{\operatorname{senh}^6 kd}$$

$$c^2 = \frac{2L}{3\pi} \operatorname{tanh} kd \left[ 1 + \left(\frac{\pi a}{L}\right)^2 \frac{8 + \cosh 4kd}{8 \operatorname{senh}^4 kd} \right]$$

Las aproximaciones son para  $\frac{1}{10} \leq \frac{d}{L} \leq \frac{1}{8}$



### 2° Aproximación $\theta = kx - \sigma t$

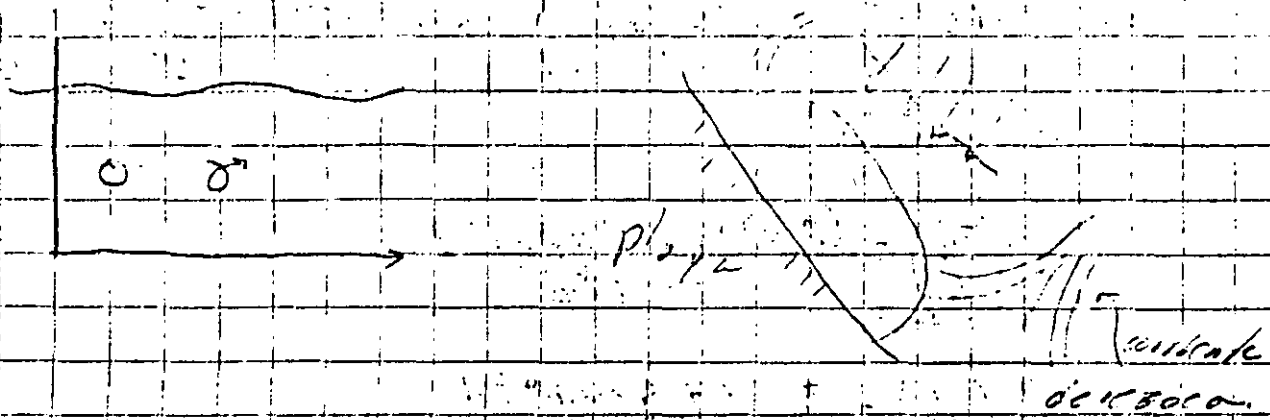
$$\xi_2 = -\frac{HgT^2}{4\pi L} \frac{\cosh k(z+d) \operatorname{sen} \theta}{\cos kd} + \frac{\pi H^2}{8L \operatorname{senh}^2 kd} \left\{ \frac{1}{2} \frac{\cosh 3k(z+d)}{\operatorname{sen}^2 kd} \right\}$$

$$+ \left(\frac{\pi H}{L}\right) \frac{c t}{2} \frac{\cosh 2k(z+d)}{\operatorname{senh}^2 kd}$$

↑ ¡ES UN TERMINO NO PERIODICO!

La distancia desplazada durante un tiempo igual al periodo  $T$  dividido entre  $T$  se conoce como transporte de masa

Corrientes de retorno o de resaca



### ★ Transporte de masa

Desplazamiento

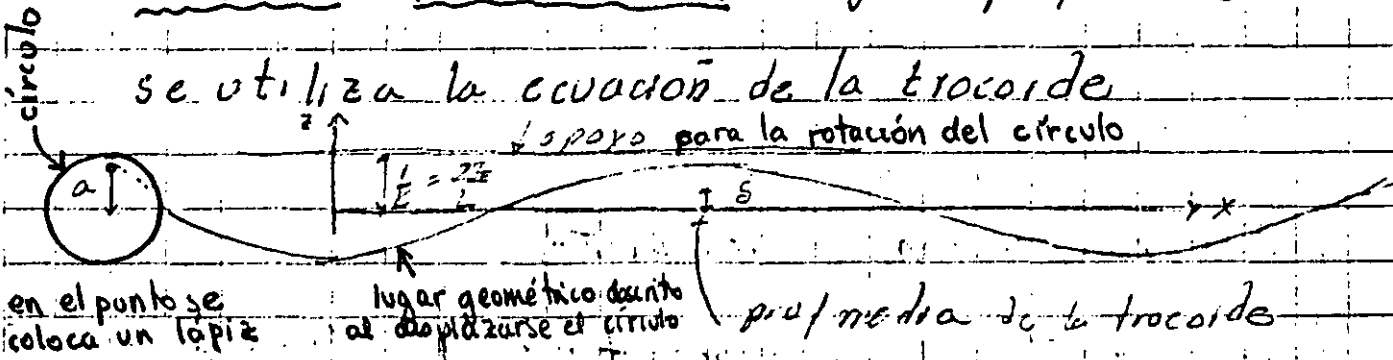
$$U(z) = \left(\frac{\pi H}{L}\right) \frac{LX \cosh 2k(z+d)}{2 \sinh^2 kd}$$



### 1.3 Otras teorías de ondas

- Teoría TROICOIDAL (aguas profundas)

se utiliza la ecuación de la trocoide

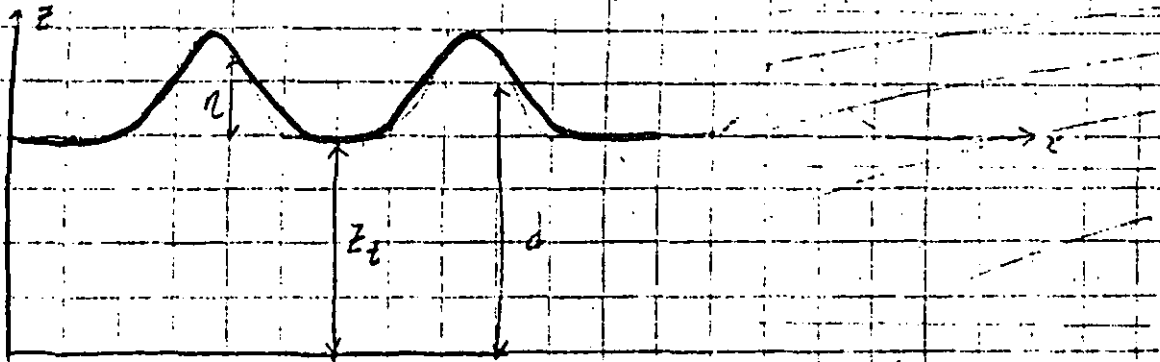


$$S = \frac{\pi H^2}{4L}$$

$S$  es la que hay entre el eje de las abscisas y la profundidad media

## • Teoría de las ondas cnoidales

sirve para aguas poco profundas e intermedias.

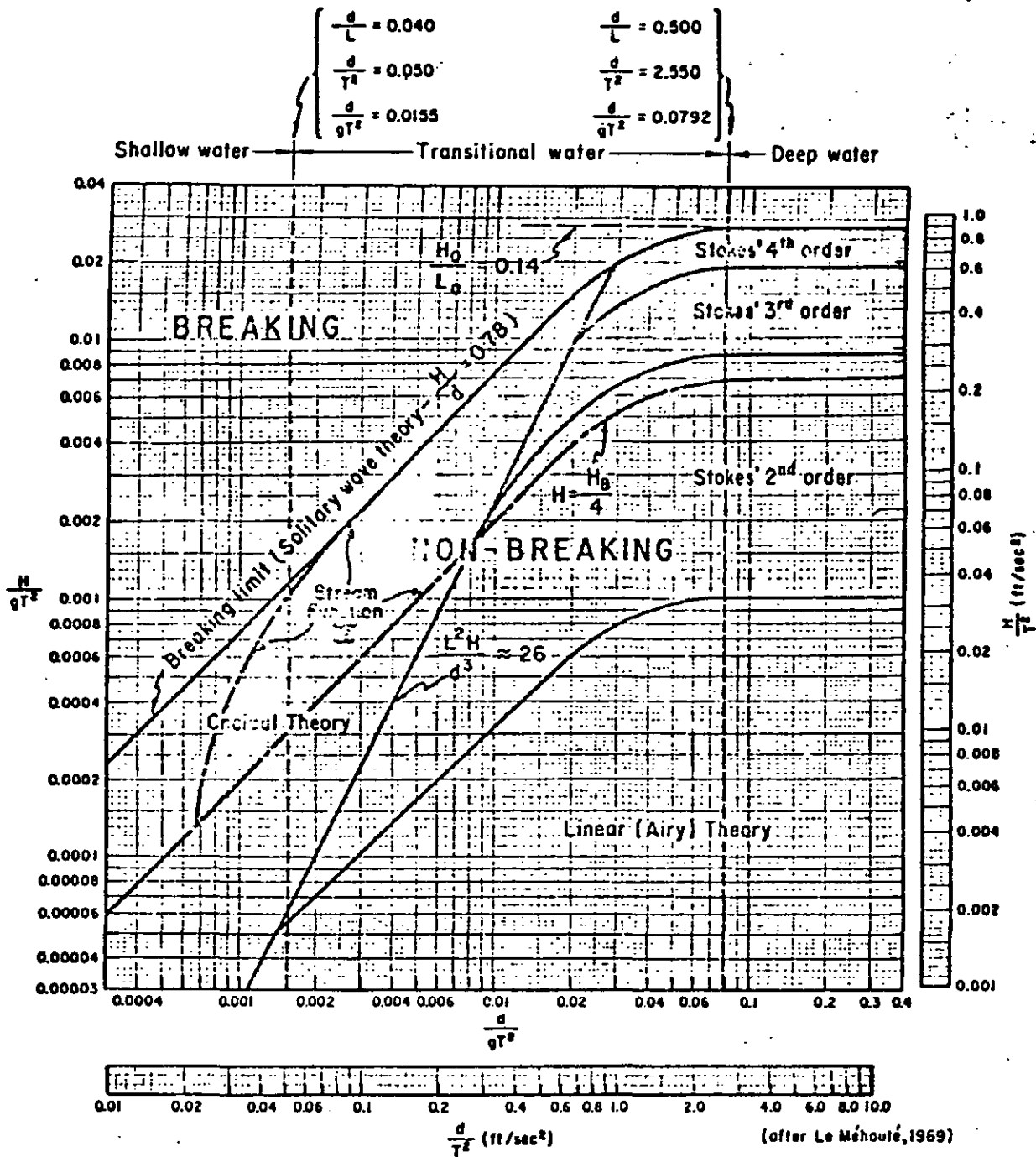


$$\eta = z_b + H \operatorname{cn}^2 \left[ \frac{2kL}{T} (x - ct), k \right]$$

$\operatorname{cn}(\cdot)$ : función coseno cnoidal.

$$K(k) = \int_0^{\pi/2} \frac{dx}{\sqrt{1 - k^2 \sin^2 x}} \quad \text{integral elíptica}$$

# Regiones de aplicación de las teorías de oleaje

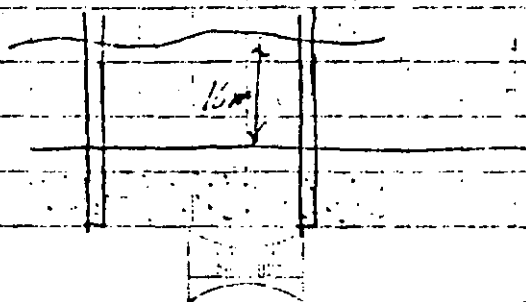




## EJEMPLOS

Ejemplo 1.

Se desea determinar si las pilas de un muelle se verán afectadas por la erosión cuando se presentan olas de 4 m de altura y 8 seg. La profundidad del fondo es 16 m y el material del mismo resiste hasta velocidades de 2.5 m/seg.



La solución consiste en encontrar la velocidad horizontal  $u$  en el fondo.

$u$  es máximo cuando  $\cos(kx - \omega t) = 1$

Datos:  $H = 4 \text{ m}$ ,  $T = 8 \text{ seg}$ ,  $d = 16 \text{ m}$ ,  $V = 2.5 \text{ m/seg}$ ,

solución.

$$L_0 = \frac{g T^2}{2\pi} = \frac{9.81(8)^2}{2\pi} = 100 \text{ m} \quad (\text{para aguas profundas})$$

$$\text{con } \frac{d}{L_0} = \frac{16}{100} = 0.16 \Rightarrow \frac{d}{L} = 0.1917$$

$$L = \frac{16}{0.1917} = 83.46 \text{ m}$$

$$u = \frac{kg a}{\omega} \frac{\cosh k(z+d)}{\cosh kd} \cos(kx - \omega t)$$

$$\text{para } z = -d \quad u = \frac{kg a}{\omega} \frac{1}{\cosh kd}$$

$x = t = 0$

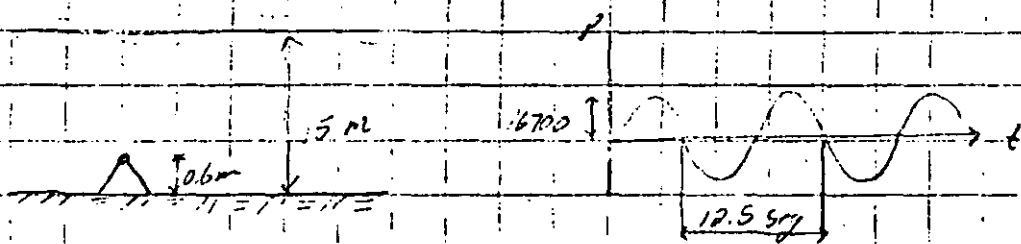
$$k = \frac{2\pi}{L} = \frac{2\pi}{83.46} = 0.0753; \quad \omega = \frac{2\pi}{T} = \frac{2\pi}{8} = 0.7854$$

$$\cosh kd = \cosh(0.0753)(16) = 1.817$$

$$u_{\max} = \frac{0.0753(9.81)^2}{0.7854} \frac{1}{1.817} = 1.035 \text{ m/seg}$$

como  $u_{\max} < u_{\text{res}}$  (no hay erosión)

2.- Un traductor de presión localizado a 0.6 m del fondo en un lugar donde la profundidad del agua es 15 m. El aparato ha registrado una presión máxima de 16700 kg/m<sup>2</sup> con una frecuencia de 0.08 seg<sup>-1</sup>. Encuentre la altura de la ola.



$$P = \gamma a \frac{\cosh k(z+d) \cos(kx - \omega t)}{\cosh kd} - \gamma z$$

$$L_0 = 1.561 (12.5)^2 = 243.95 \text{ m}$$

$$\frac{d}{L_0} = \frac{15}{243.95} = 0.0615 \quad \frac{d}{L} = 0.1052 \quad ; \quad L = 141.78 \text{ m}$$

$$k = \frac{2\pi}{L} = \frac{2\pi}{141.78} = 0.0443$$

$$16700 = 1013 \frac{H}{2} \frac{\cosh(0.0443)(-14.4+15)}{\cosh(0.0443)(15)} - 1013(-14.4)$$

$$H = \frac{16700 - (1013)(14.4)}{1013} \frac{2}{1.0003} = 5.63 \text{ m}$$

El factor de respuesta de la presión es

$$K = \frac{\cosh(kxz+d)}{\cosh kd} = \frac{1.0003}{1.2248} = 0.817$$

3.- A una profundidad de 30 m se tiene un oleaje de periodo de 12 seg. y altura 5 m. Encuentre a) la longitud de la ola, b) la celeridad, c) la energía total por unidad de longitud de cresta

Datos:  $d = 30 \text{ m}$ ,  $T = 12 \text{ seg}$ ,  $H = 5 \text{ m}$

Solución:

a) Como no se sabe si se está en la condición de aguas profundas o no, se utiliza la siguiente expresión y se resuelve por aproximaciones sucesivas (es convergente y recursiva)

$$L = \frac{g T^2}{2\pi} \tanh \frac{2\pi d}{L}$$

se propone  $L = 177.04 \text{ m}$

$$L = \frac{9.81 (12)^2}{2\pi} \tanh \frac{2\pi (30)}{177.04} = 177.04 \text{ m}$$

$$\therefore \boxed{L = 177.04 \text{ m}} \checkmark$$

como  $\frac{d}{L} = \frac{30}{177.04} = 0.17$  y  $0.5 > 0.17 > 0.04$

se está en aguas intermedias.

$$b) \quad c = \sqrt{\frac{g}{k} \tanh kd} = \sqrt{\frac{gL}{2\pi} \tanh \frac{2\pi d}{L}}$$

$$c = \sqrt{\frac{9.81 (177.04)}{2\pi} \tanh \frac{2\pi (30)}{177.04}}$$

$$\therefore \boxed{c = 14.75 \frac{\text{m}}{\text{seg}}} \checkmark$$

otra manera sencilla de resolver el segundo inciso es

$$c = \frac{L}{T} = \frac{177.04}{12} = 14.75 \text{ m/seg}$$

c) con  $\gamma = 1.025 \text{ ton/m}^3$  y  $a = H/2 = 5/2 = 2.5 \text{ m}$

$$E_T = \frac{\gamma a^2 L}{2} = \frac{(1.025)(2.5)^2 (177.04)}{2}$$

$$E_T = 567.09 \frac{\text{ton} \cdot \text{m}}{\text{m}} \checkmark$$

4. Sea una onda de 300m de largo y 10m de altura, donde la profundidad del fondo es 150m. Calcule:  
a) la celeridad de la onda, b) los desplazamientos de las partículas de agua en el fondo y c) compare la celeridad con la de aguas poco profundas.

Datos:  $L = 300 \text{ m}$ ;  $H = 10 \text{ m}$ ,  $d = 150 \text{ m}$

Solución:

$$\frac{d}{L} = \frac{150}{300} = 0.5 \text{ se esta en aguas profundas (Lim.)}$$

$$a) c_0 = \sqrt{\frac{g L}{2\pi}} = \sqrt{\frac{9.81(300)}{2\pi}} = 21.64 \text{ m/seg}$$

$$\therefore c_0 = 21.64 \text{ m/seg} \checkmark$$

$$b) \zeta_y = -a e^{kz} \text{sen} \theta = -\frac{H}{2} e^{\frac{2\pi d}{L}} \text{sen}(kx - \pi t)$$

(para desplazamiento horizontal)

como  $c_0 = \frac{L}{T_0}$  ;  $T_0 = \frac{L}{c_0} = \frac{300}{21.64}$

$T_0 = 13.86 \text{ seg}$

para desplazamiento vertical

$\xi = a e^{kz} \cos \theta = \frac{H}{2} e^{\frac{-2\pi d}{L}} \cos(kx - \omega t)$

$k = \frac{2\pi}{L} = \frac{2\pi}{300} = 0.0209$  ;  $\omega = \frac{2\pi}{T} = \frac{2\pi}{13.86} = 0.4533$

para  $x = 0$

t (seg)	$\xi$ (m)	$\zeta$ (m)
T/4	0.2161	0.0
T/2	0.0	-0.2161
3/4 T	-0.2161	0.0
T	0.0	0.2161

c) para aguas poco profundas

$\frac{d}{L} < \frac{1}{25}$  ;  $d < \frac{300}{25} = 12 \text{ m}$

$c = \sqrt{gd} = \sqrt{9.81(12)}$

$c = 10.85 \text{ m/seg}$

$\therefore c_0 > c$

5. ¿Cuales son: a) las velocidades orbitales y b) los desplazamientos máximos a una profundidad de 10m, de un oleaje de 8seg., 4 m de altura y donde la profundidad del fondo está a 20m

Datos:  $T = 8 \text{ seg}$ ,  $H = 4 \text{ m}$ ,  $d = 20 \text{ m}$ ,  $z = -10 \text{ m}$

Solución:

Cálculo de la longitud  $L$

$$L = \frac{gT^2}{2\pi} \tanh \frac{2\pi d}{L}$$

$$L = \frac{9.81(8)^2}{2\pi} \tanh \frac{2\pi(20)}{L}$$

$$L = 88.79 \text{ m} \checkmark$$

$$\frac{d}{L} = \frac{20}{88.79} = 0.23$$

como  $0.5 > 0.23 > 0.04$

se está en aguas intermedias.  $\checkmark$

a) Cálculo de la velocidad horizontal para  $z = -10 \text{ m}$

$$u = \frac{kg a \cosh[k(z+d)]}{\sigma \cosh kd} \cos(kx - \sigma t)$$

$$k = \frac{2\pi}{L} = \frac{2\pi}{88.79} = 0.07; \quad \sigma = \frac{2\pi}{T} = \frac{2\pi}{8} = 0.79; \quad a = \frac{H}{2} = 2 \text{ m}$$

La velocidad vertical de la partícula es

$$w = \frac{kga}{\sigma} \frac{\sinh[k(z+d)]}{\cosh kd} \sin(kx - \sigma t)$$

para  $\begin{cases} z = -10 \text{ m} \\ x = 0 \text{ m} \end{cases}$

$t$ (seg)	$u$ (m/seg)	$w$ (m/seg)
$T/4$	0.0	-0.6229
$T/2$	-1.0316	0.0
$3T/4$	0.0	0.6229
$T$	1.0316	0.0

b) Cálculo de los desplazamientos máximos

- desplazamiento horizontal

$$\xi = -a \frac{\cosh[k(z+d)]}{\sinh kd} \sin(kx - \tau t)$$

es máximo cuando  $\sin(kx - \tau t) = \pm 1$

$$\xi_{\text{máx}} = -2 \frac{\cosh[0.07(-10+20)]}{\sinh(0.07)(20)} (\pm 1) = \pm 1.30 \text{ m} \checkmark$$

- desplazamiento vertical

$$\xi = a \frac{\sinh[k(z+d)]}{\sinh kd} \cos(kx - \tau t)$$

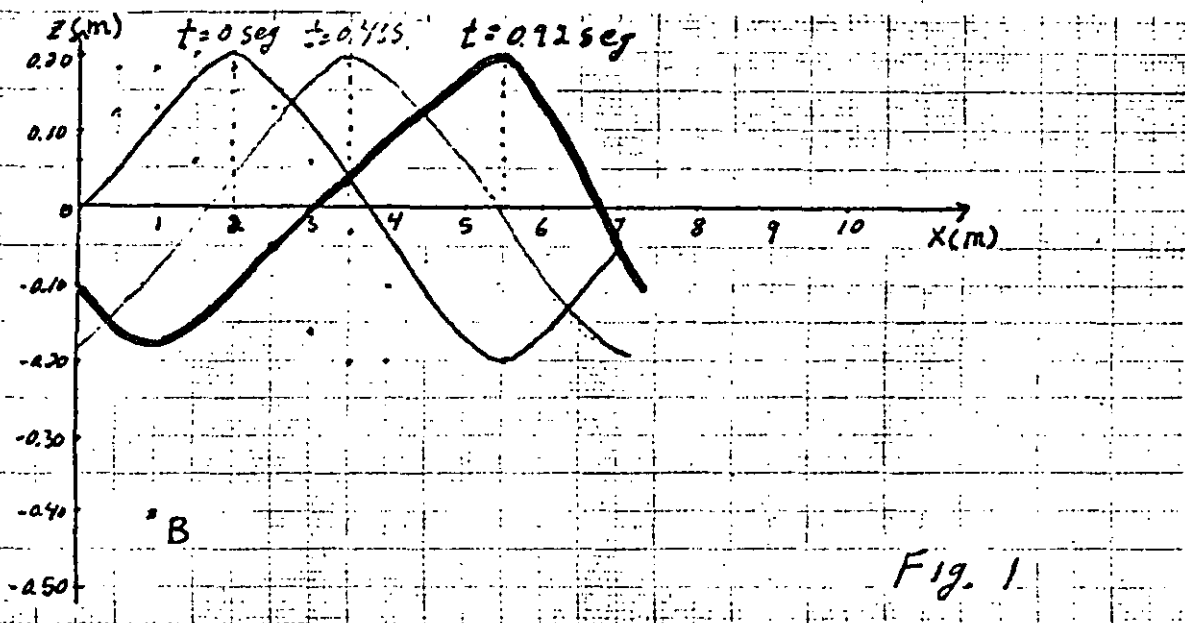
es máximo cuando  $\cos(kx - \tau t) = \pm 1$

$$\xi_{\text{máx}} = 2 \frac{\sinh[0.07(-10+20)]}{\sinh(0.07)(20)} (\pm 1) = \pm 0.7930 \text{ m} \checkmark$$

6. En un canal de oleaje monocromático se ha obtenido por medio de mediciones la información contenida en la fig 1. En la línea punteada marcada con  $t=0$  s. se señala el punto donde empezó a funcionar el cronómetro. Si la profundidad del agua es de 1.6 m. Determine: a) la celeridad, periodo y frecuencia del oleaje. b) las aceleraciones de las partículas de agua. c) la expresión matemática para la superficie de la ola. d) la potencia



promedio por unidad de longitud de cresta. e) La presión en el punto B ubicado a  $x=1\text{ m}$  y  $z=-0.4\text{ m}$  (fig 1), para  $t=0$ ,  $t=0.46\text{ seg}$ ,  $t=0.92\text{ seg}$ . f) compare los resultados de e) con respecto con las presiones cuando no hay oleaje, desde luego para el mismo punto B.



Datos:  $d = 1.6\text{ m}$

a) de la figura (1)

$$a = 0.20\text{ m} ; H = 2a = 0.40\text{ m}$$

$$L \approx 7.5\text{ m} ; \frac{0.46\text{ seg}}{1.5\text{ m}} = \frac{T}{7.5} \quad T = 2.3\text{ seg.}$$

solución

a) la celeridad se puede calcular con la siguiente fórmula

$$c = \sqrt{\frac{g}{k} \tanh kd} \quad ; \quad k = \frac{2\pi}{L}$$

$$c = \sqrt{\frac{9.81}{2\pi} \tanh \frac{2\pi d}{L}} = \sqrt{\frac{9.81(7.5)}{2\pi} \tanh \frac{2\pi(1.6)}{7.5}}$$

$$\therefore c = 3.195 \text{ m/seg} \quad \checkmark$$

$$\text{como } c = \frac{L}{T} \quad ; \quad T = \frac{L}{c} = \frac{7.5}{3.195} = \dots \quad \therefore T = 2.347 \text{ seg} \quad \checkmark$$

$$\text{y la frecuencia } f \text{ es } f = \frac{1}{T} = \frac{1}{2.347}$$

$$f = 0.426 \text{ seg}^{-1} \quad \checkmark$$

Revisión se debe cumplir que

$$L = \frac{gT^2}{2\pi} \tanh \frac{2\pi d}{L}$$

$$7.5 = \frac{9.81(2.347)^2}{2\pi} \tanh \frac{2\pi(1.6)}{7.5} \quad ; \quad 7.5 \approx 7.499 \quad (\text{si cumple}) \quad \checkmark$$

También se observa que el periodo obtenido de la figura y el periodo obtenido con las expresiones es aproximadamente igual

$$T_o = 2.3 \quad (\text{observado}) \quad \checkmark$$

$$T_e = 2.347 \text{ seg} \quad (\text{estimado}) \quad \checkmark$$

b) Las aceleraciones de las partículas de agua en

$$z = -d/2 = -0.80 \text{ m son}$$

- dirección horizontal

$$a_x = kga \frac{\cosh[k(z+d)]}{\cosh kd} \sin(kx - \omega t)$$

$$d = 1.6 \text{ m}; k = \frac{2\pi}{L} = 0.8377; z = -0.8 \text{ m} \quad T = 2.347 \text{ seg}$$

- dirección vertical

$$a_z = -kga \frac{\sinh[k(z+d)]}{\cosh kd} \cos(kx - \omega t)$$

para  $x=0$

t	$a_x$	$a_z$
T/4	-0.9930	0.0
T/2	0.0	0.5810
3/4 T	0.9930	0.0
T	0.0	-0.5810

c) La expresión es

$$\eta = 0.20 \cos 2\pi(x/1.5 - t/2.347)$$

para  $t=0$

x	$\eta$
0	0.20
0.5	0.18
1	0.13
1.5	0.06
2	-0.02
2.5	-0.10
3	-0.16
3.5	-0.20
4	-0.20

para  $t=0.46 \text{ seg}$

x	$\eta$
0	0.06
0.5	0.13
1	0.18
1.5	0.20
2	0.18
2.5	0.13
3	0.06
3.5	-0.03
4	-0.10

ver fig 1

d) La potencia promedio por unidad de longitud de cresta

$$\bar{P} = \frac{\rho E}{T} = \frac{\gamma A^2 L}{2 T} \frac{1}{2} \left( 1 + \frac{2kd}{\sinh 2kd} \right)$$

$$\bar{P} = \frac{(1.025)(0.20)^2 (7.5)}{2 \cdot 2.347} \frac{1}{2} \left( 1 + \frac{2(2\pi/7.5)1.6}{\sinh 2(2\pi/7.5)1.6} \right)$$

$$\therefore \bar{P} = 0.0 \quad \frac{\text{ton m}}{\text{m}^2 \text{seg}}$$

e) La presión en el punto B según t es

$$p(t) = \gamma \eta \frac{\cosh[k(z+d)]}{\cosh kd} - \gamma z$$

$$p(0) = 1.025 (0.2) \cos\left(\frac{2\pi}{7.5}(1) - \frac{2\pi}{2.347}0\right) \frac{\cosh \frac{2\pi}{7.5}(-0.4+1.6)}{\cosh \frac{2\pi}{7.5}(1.6)} - 1.025(-0.40) =$$

$$p(0) = 0.5141 \text{ ton/m}^2$$

$$p(0.46) = 1.025 (0.2) \cos\left(\frac{2\pi}{7.5}(1) - \frac{2\pi}{2.347}(0.46)\right) \frac{\cosh \frac{2\pi}{7.5}(-0.4+1.6)}{\cosh \frac{2\pi}{7.5}(1.6)} - 1.025(-0.40) =$$

$$p(0.46) = 0.5537 \text{ ton/m}^2$$

$$p(0.92) = 1.025 (0.2) \cos\left(\frac{2\pi}{7.5}(1) - \frac{2\pi}{2.347}(0.92)\right) \frac{\cosh \frac{2\pi}{7.5}(-0.4+1.6)}{\cosh \frac{2\pi}{7.5}(1.6)} - 1.025(-0.40) =$$

$$p(0.92) = 0.4015 \text{ ton/m}^2$$

f) La presión en B cuando no hay oleaje es

$$p = \gamma z = 1.025(0.40) = 0.41 \text{ ton/m}^2$$

$$p < p(0)$$

$$p \approx p(0.92)$$

$$p < p(0.46)$$

7. Demostrar que la energía total por unidad de longitud de cresta es

$$E = E_k + E_p = \frac{\gamma H^2 L}{16} + \frac{\gamma H^2 L}{16} = \frac{\gamma H^2 L}{8} \quad (1)$$

- Cálculo de  $E_c$

$$E_c = \frac{\gamma}{2g} \int_0^L \int_{-d}^0 (u^2 + w^2) dx dz \quad (2)$$

$$\text{donde } u^2 = (kga)^2 \frac{\cosh^2 k(z+d)}{\cosh^2 kd} \sin^2(kx - \tau t) \quad \dots (a)$$

$$w^2 = (-kga)^2 \frac{\cosh^2 k(z+d)}{\cosh^2 kd} \cos^2(kx - \tau t) \quad (b)$$

sustituyendo a y b en 2 queda

$$E_c = \frac{\gamma}{2g} \int_0^L \int_{-d}^0 \left[ \frac{(kga)^2 \cosh^2 k(z+d)}{\tau^2 \cosh^2 kd} \sin^2(kx - \tau t) + \frac{(-kga)^2 \cosh^2 k(z+d)}{\cosh^2 kd} \cos^2(kx - \tau t) \right] dx dz$$

$$E_c = \frac{\gamma}{2g} \int_0^L \int_0^d \left( \frac{gka}{v} \right)^2 \frac{1}{\cosh^2 kd} \left[ \cos^2 h k(z+td) \cos^2(kx-vt) + \sinh^2 k(z+td) \sin^2(kx-vt) \right] dz dx$$

$$E_c = \frac{\gamma}{2g} \left( \frac{gka}{v} \right)^2 \frac{1}{\cosh^2 kd} \frac{\pi}{k^2} (\sin kd \cos kd)$$

y como  $v^2 = kg \tanh kd$

$$\therefore E_c = \frac{\gamma a^2}{4} L$$

= Cálculo de  $E_p$

$$E_p = \gamma \int_0^L \int_0^{\eta} z dx dz = \gamma \int_0^L \frac{z^2}{2} \Big|_0^{\eta} dx = \gamma \int_0^L \frac{\eta^2}{2} dx$$

Como  $\eta = a \cos(kx - vt)$

$$E_p = \frac{\gamma}{2} \int_0^L a^2 \cos^2(kx - vt) dx = \frac{\gamma a^2}{2} \left( \frac{\pi}{k} \right)$$

como  $k = 2\pi/L$

$$E_p = \frac{\gamma a^2}{4} L$$

Recordando que  $E_T = E_c + E_p$

finalmente

$$E_T = \frac{\gamma a^2}{2} L$$

8.: Demostrar que la potencia por unidad de longitud de cresta es

$$\bar{P} = \frac{\rho E}{T}$$

Se define como potencia de una onda a la energía de la onda por unidad de tiempo propagada en la dirección de recorrido de la onda.

La potencia es  $P = \int_0^{-d} (\rho + \gamma z) u dz$

pero  $\rho + \gamma z = \gamma \eta \frac{\cosh k(z+d)}{\cosh kd}$

y  $\eta = a \cos(kx - \omega t)$

$$P = \int_0^{-d} \left[ \gamma a \frac{\cosh k(z+d)}{\cosh kd} \cos(kx - \omega t) \right] \left[ \frac{g a k \cos k(z+d)}{\omega \cosh kd} \cos(kx - \omega t) \right] dz$$

$$P = \frac{\gamma a^2 g}{\omega} \frac{\cos^2(kx - \omega t)}{\cosh^2 kd} \int_0^{-d} \cosh^2 k(z+d) k dz$$

$$P = \frac{\gamma a^2 g}{\omega} \frac{\cos^2(kx - \omega t)}{\cosh^2 kd} \left[ \frac{k d}{2} + \frac{\sinh 2kd}{4} \right]$$

La potencia promedio durante  $T$  es  $\bar{P}$

$$\bar{P} = \frac{1}{T} \int_0^T P dt$$

$$\bar{P} = \frac{1}{T} \frac{\gamma a^2 g}{\cosh^2 kd} \left[ \frac{kd}{2} + \frac{\sinh 2kd}{4} \right] \int_0^T \cos^2(kx - \omega t) \omega dt$$

$$\bar{P} = \frac{1}{T} \frac{\gamma a^2 g}{\cosh^2 kd} \left[ \frac{kd}{2} + \frac{\sinh 2kd}{4} \right] \pi$$

recordando que  $\frac{g}{\omega} = \frac{c}{\tanh kd}$

$$\bar{P} = \frac{1}{T} \frac{\gamma a^2 g}{\tanh kd} \frac{\pi}{\cosh^2 kd} \left[ \frac{kd}{2} + \frac{\sinh 2kd}{4} \right]$$

$$\bar{P} = \frac{\gamma a^2 c}{\sinh 2kd} \left[ \frac{kd}{2} + \frac{\sinh 2kd}{4} \right]$$

$$\bar{P} = \gamma a^2 c \frac{1}{4} \left[ 1 + \frac{2kd}{\sinh 2kd} \right]$$

$$\bar{P} = \frac{\gamma a^2}{2} \frac{L}{T} \frac{1}{2} \left[ 1 + \frac{2kd}{\sinh 2kd} \right]$$

como  $\frac{cg}{c} = n = \frac{1}{2} \left[ 1 + \frac{2kd}{\sinh 2kd} \right]$  y  $E = \frac{\gamma a^2 L}{2}$

finalmente

$$\bar{P} = \frac{n E T}{T}$$